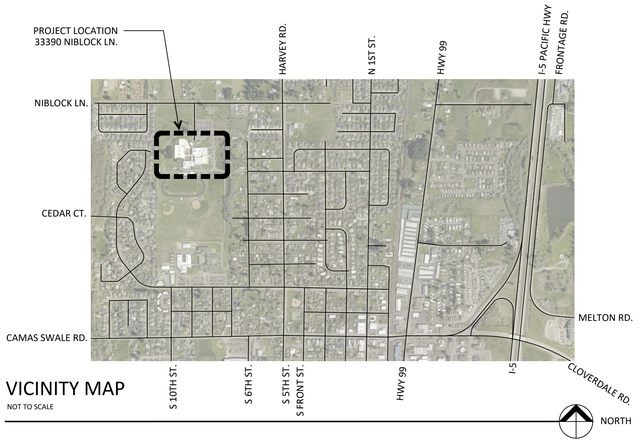
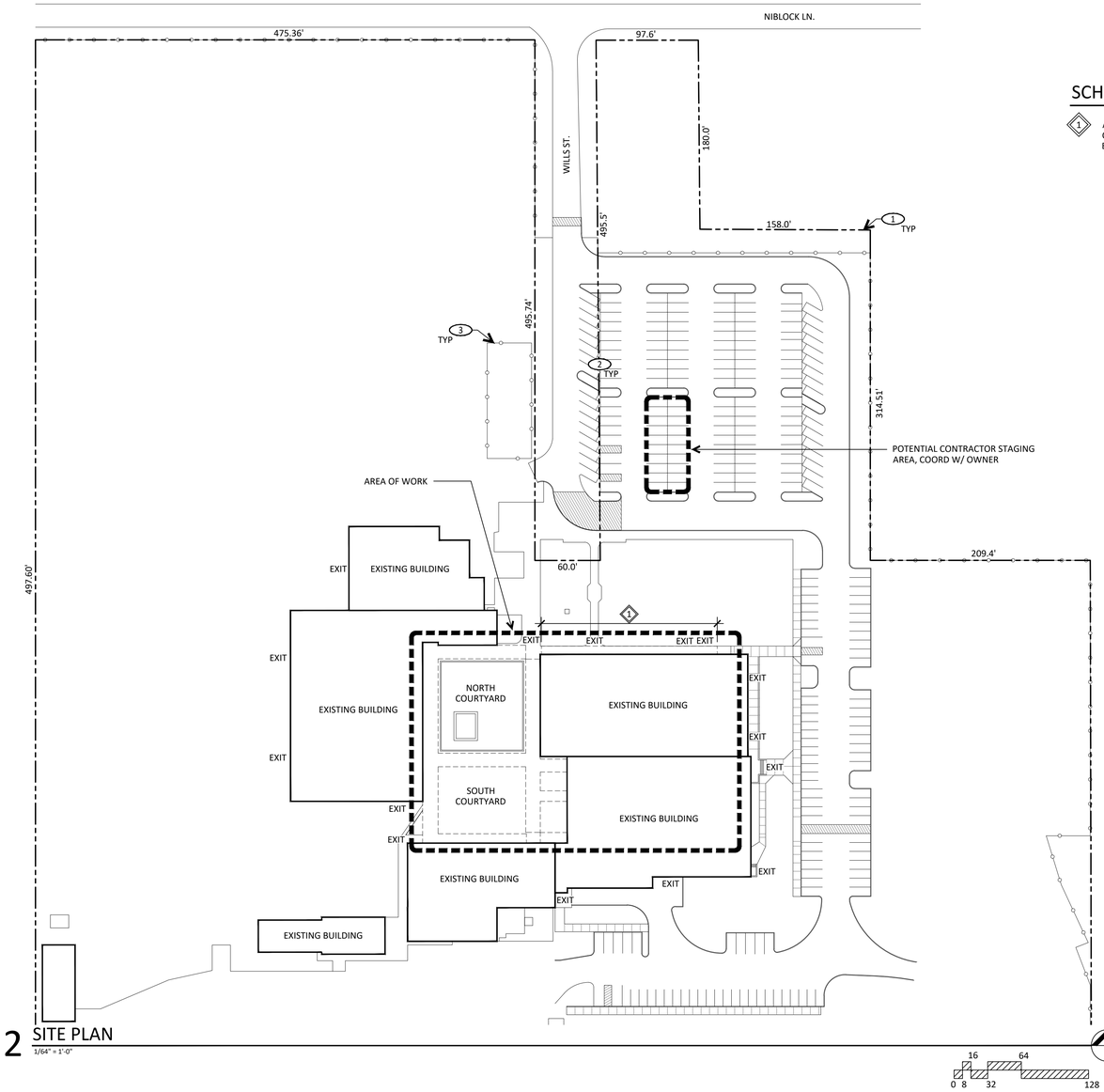


1 VICINITY MAP

NOT TO SCALE



2 SITE PLAN

1/64" = 1'-0"

PROJECT INFORMATION

SITE ADDRESS
33390 NIBLOCK LN, CRESWELL, OR 97426

MAP + TAXLOT
1903151100100

OWNER
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(541) 895-6000
POC: JOEL HIGDON
EMAIL: JHIGDON@CRESWELL.K12.OR.US

ARCHITECT
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KPFF
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POC: TRAVIS SMITH
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SUMMARY OF WORK & CODE ANALYSIS

SUMMARY OF WORK
ALTERATIONS TO AND REPLACEMENT OF CANOPIES AT EXISTING HIGH SCHOOL. WORK INCLUDES SELECTIVE DEMOLITION AND NEW CONCRETE PAVING AND STEEL REINFORCED FOOTINGS, METAL FABRICATIONS, ROUGH FRAMING, SIDING, SHEET METAL, ROOFING, PAINTING, FIRE SPRINKLERS, ELECTRICAL, AND EARTHWORK.

NO NEW BUILDING AREA.

ELECTRICAL DESIGN-BUILD
FIRE SPRINKLERS DESIGN-BUILD

DEFERRED SUBMITTALS:
FIRE SPRINKLERS
ELECTRICAL

SEE STRUCTURAL FOR REQ'D SPECIAL INSPECTIONS

2022 OREGON STRUCTURAL SPECIALTY CODE

CHAPTER 3 - USE & OCCUPANCY
GROUP E OCCUPANCY

PER 312.1 BUILDINGS AND STRUCTURES OF AN ACCESSORY CHARACTER AND MISCELLANEOUS STRUCTURES NOT CLASSIFIED IN ANY SPECIFIC OCCUPANCY SHALL BE CONSTRUCTED, EQUIPPED AND MAINTAINED TO CONFORM WITH THE REQUIREMENTS OF THIS CODE COMMENSURATE WITH THE FIRE AND LIFE HAZARD INCIDENTAL TO THEIR OCCUPANCY.

CHAPTER 5 - GENERAL BUILDING HEIGHTS & AREAS
NO CHANGES TO EXISTING BUILDING AREA
TOTAL GROSS AREA: 84,235 SF
AREA OF CANOPY ASSOCIATED W/ BUILDING: 10,080 SF

(E) BUILDING HEIGHT: 34'-4"
ONE STORY STRUCTURE

CHAPTER 6 - TYPES OF CONSTRUCTION
CANOPY CONSTRUCTION TYPE 2-B

PER TABLE 601, NON-RATED STEEL PRIMARY FRAME AND ROOF CONSTRUCTION

CHAPTER 9 - FIRE PROTECTION SYSTEMS
PER TABLE 601, NON-RATED STEEL PRIMARY FRAME AND ROOF CONSTRUCTION

CHAPTER 10 - MEANS OF EGRESS
NO CHANGE TO BUILDING OCCUPANT LOAD

PER 1008.3, EGRESS LIGHTING W/ EMERGENCY POWER SOURCE REQ'D

CHAPTER 34 - EXISTING BUILDINGS
ACCESSIBILITY & BARRIER REMOVAL PER ORS 447.241
ADA COMPLIANT FACILITIES EXISTING
- PARKING
- ENTRANCE
- ROUTE
- RESTROOMS
- TELEPHONES
- DRINKING FOUNTAINS
- STORAGE

SCHEDULE OF ALTERNATES

1 ALTERNATE NO. ONE: NORTH CANOPY. ADD ENTIRETY OF WORK ASSOCIATED WITH THE CANOPY NORTH OF EXISTING GYMNASIUM.

GENERAL NOTES

1. DIMENSIONS ARE APPROXIMATE - FIELD VERIFY PRIOR TO WORK, COORDINATE W/ ARCH.
2. PROVIDE FIRE SPRINKLERS THROUGHOUT NEW CANOPIES AS REQ'D BY AUTHORITIES HAVING JURISDICTION, PAINT PIPING TO MATCH PROJECT STD.
3. DEMOLITION DRAWINGS ARE DIAGRAMMATIC & DESCRIBE GENERAL INTENT - PERFORM WORK AS REQ'D TO PREPARE FOR & EXECUTE NEW WORK, WRECK OUT ABANDONED CABLING AND BUILDING SYSTEMS. BRING DISCREPANCIES TO ARCHITECT'S ATTENTION PRIOR TO WORK.
4. PROVIDE EMERGENCY LIGHTING W/ INITIAL, MINIMUM, AND AVERAGE ILLUMINATION IN ACCORDANCE W/ OSSC 1008.3.5, SEE DRAWINGS FOR PATHWAY - CONNECT TO EXISTING EMERGENCY ELECTRICAL POWER SUPPLY.

SHEET INDEX

EXISTING CONDITIONS PHOTOS

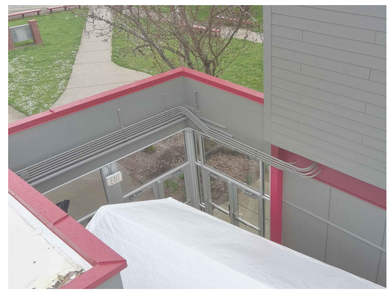

NORTH COURTYARD - LOOKING SOUTH


NORTH COURTYARD - LOOKING EAST


SOUTH COURTYARD - LOOKING EAST


NORTH OF GYMNASIUM - LOOKING EAST


NORTH OF GYMNASIUM - LOOKING WEST


NORTH ENTRY - LOOKING NORTH

KEYNOTES

1 LINE TYPE INDICATES PROPERTY LINE
2 (E) PARKING
3 (E) FENCE

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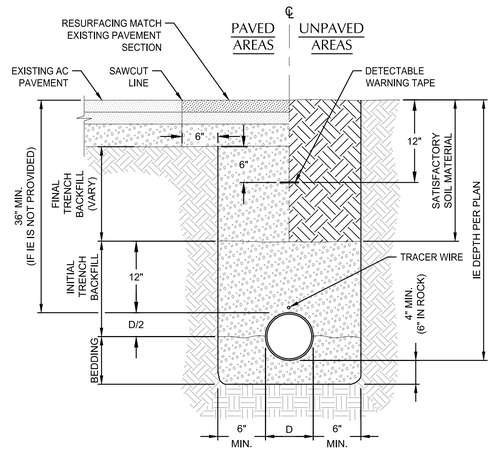
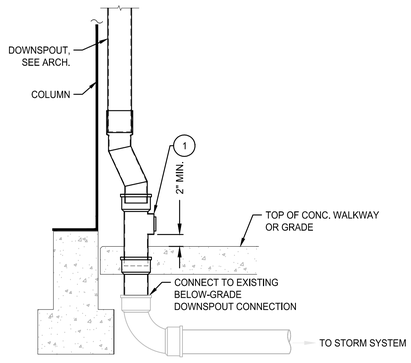
COVER

G0.10



1 TYPICAL PIPE BEDDING AND BACKFILL

SCALE: NTS

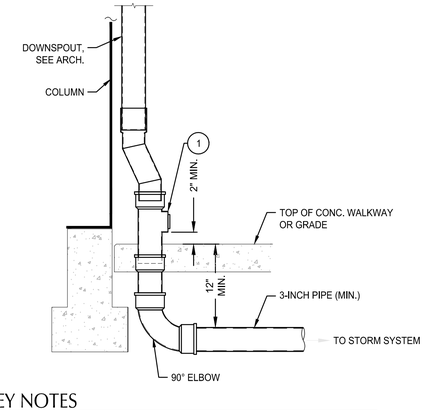


(X) KEY NOTES

1 CAST-IRON OR DUCTILE IRON CLEANOUT TEE WITH COUNTERSUNK PLUG. DIAMETER SIZE TO MATCH SIZE ON PLANS. PROVIDE SUBMITTAL FOR APPROVAL BY ENGINEER/OWNER.

2 EXISTING DOWNSPOUT CONNECTION

SCALE: NTS

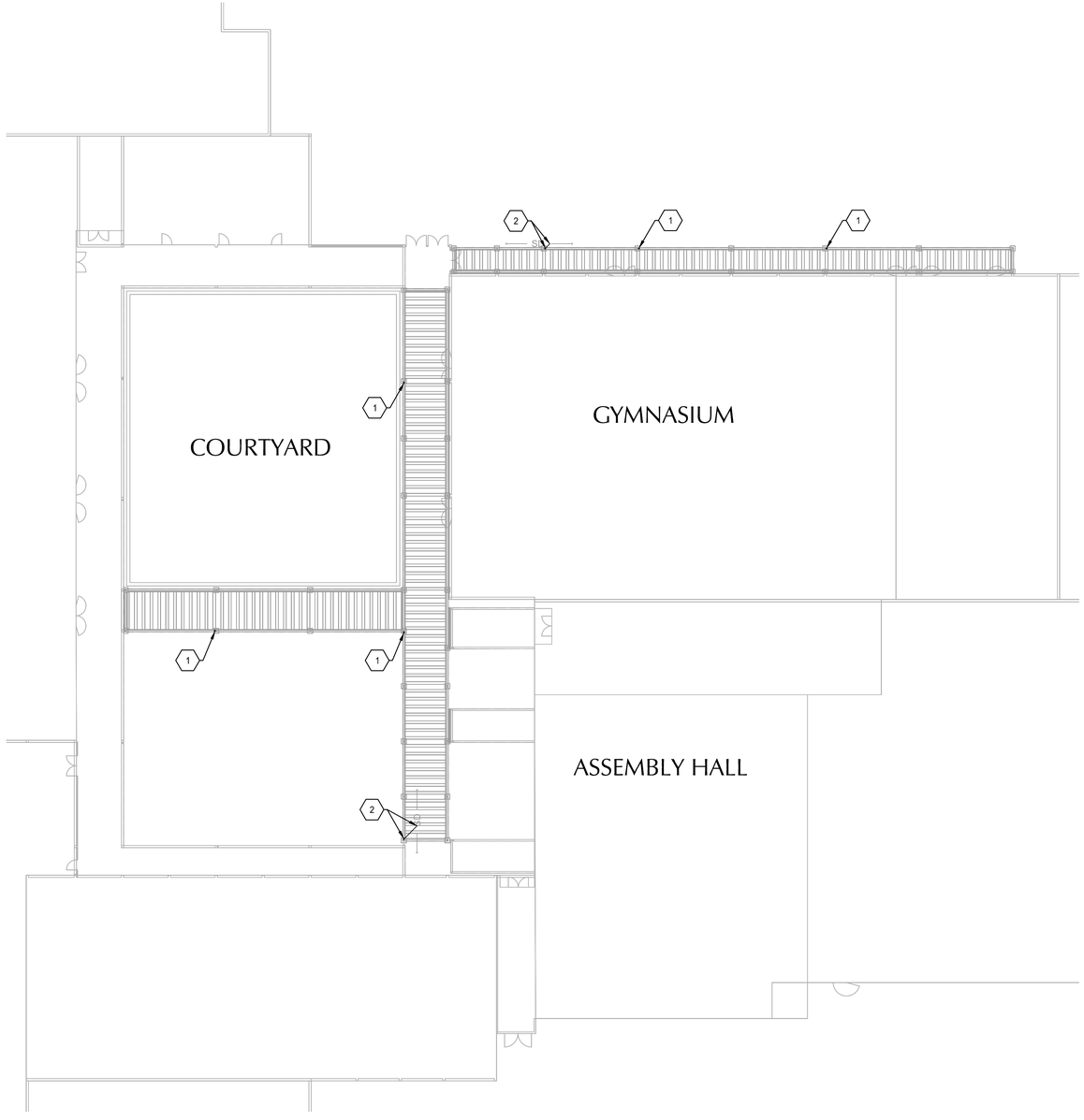


(X) KEY NOTES

1 CAST-IRON OR DUCTILE IRON CLEANOUT TEE WITH COUNTERSUNK PLUG. DIAMETER SIZE TO MATCH SIZE ON PLANS. PROVIDE SUBMITTAL FOR APPROVAL BY ENGINEER/OWNER.

3 NEW DOWNSPOUT CONNECTION

SCALE: NTS



CONSTRUCTION NOTES

EARTHWORKS

1. SUBGRADE AND TRENCH BACKFILL SHALL BE COMPACTED TO AT LEAST 95% OF THE MAXIMUM DRY DENSITY AS DETERMINED BY ASTM D698. FLOODING OR JETTING THE BACKFILLED TRENCHES WITH WATER IS NOT PERMITTED.
2. TRENCH BEDDING AND BACKFILL SHALL BE AS SHOWN ON THE PIPE BEDDING AND BACKFILL DETAIL.
3. WITH THE APPROVAL OF THE ENGINEER, PROOF-ROLL TESTING OF SUBGRADE AND/OR AGGREGATE BASE MAY BE SUBSTITUTED FOR OTHER COMPACTION TESTING.
4. CONTRACTOR SHALL PREVENT SEDIMENTS AND SEDIMENT LADEN WATER FROM ENTERING THE STORM DRAINAGE SYSTEM.

STORM AND SANITARY

1. CONNECTIONS TO EXISTING STORM AND SANITARY SEWERS SHALL CONFORM TO THE 2024 OREGON STANDARD SPECIFICATIONS FOR CONSTRUCTION, SECTION 00490, "WORK ON EXISTING SEWERS AND STRUCTURES".
2. BEGIN LAYING STORM DRAIN AND SANITARY SEWER PIPE AT THE LOW POINT OF THE SYSTEM, TRUE TO GRADE AND ALIGNMENT INDICATED WITH UNBROKEN CONTINUITY OF INVERT. THE CONTRACTOR SHALL ESTABLISH LINE AND GRADE FOR THE STORM AND SANITARY SEWER PIPE USING A LASER.
3. ALL ROOF DRAIN AND CATCH BASIN LEADERS SHALL HAVE A MINIMUM SLOPE OF 2 PERCENT UNLESS NOTED OTHERWISE IN THE PLANS.
4. ALL HORIZONTAL CONNECTIONS TO THE SANITARY OR STORM SEWERS SHALL BE OF THE 'WYE' BRANCH TYPE.

MATERIAL NOTES

1. GENERAL: MATERIALS SHALL BE NEW. THE USE OF MANUFACTURER'S NAMES, MODELS, AND NUMBERS IS INTENDED TO ESTABLISH STYLE, QUALITY, APPEARANCE, AND USEFULNESS. PROPOSED SUBSTITUTIONS WILL REQUIRE WRITTEN APPROVAL FROM ARCHITECT PRIOR TO INSTALLATION.
2. STORM AND SANITARY SEWER PIPING SHALL BE PVC PIPE (SCHEDULE 40 FOR 3-INCH OR SMALLER AND 3034 FOR 4-INCH OR LARGER). DUCTILE IRON PIPE.
3. CONCRETE FOR CURBS, SIDEWALK AND DRIVEWAYS SHALL HAVE A MINIMUM COMPRESSIVE STRENGTH OF 3,300 PSI AT 28 DAYS.
4. ALL AGGREGATE BACKFILL TO MEET THE REQUIREMENTS OF ODOT DENSE-GRADED ¾"-0".

SHEET NOTES

1. SITE NOT SURVEYED. SITE PLAN SHOWN REFLECTS RECORD DRAWINGS AND FIELD MEASUREMENTS. CONTRACTOR TO BRING ANY DISCREPANCIES TO THE ATTENTION OF THE DESIGN TEAM IMMEDIATELY UPON DISCOVERY.
2. SEE ARCHITECTURAL PLANS FOR PAVEMENT DEMOLITION AND REPAIR RELATED TO FOOTING INSTALLATION.
3. SEE ARCHITECTURAL PLANS FOR ROOF DRAIN LOCATIONS AND DOWNSPOUTS.
4. PIPE BEDDING AND BACKFILL FOR ALL UTILITIES SHALL BE DONE PER DETAIL 1/C1.

(X) UTILITY KEY NOTES

NOTE	DESCRIPTION	DETAIL REF.
1	PROPOSED DOWNSPOUT IS LOCATED TO MATCH EXISTING DOWNSPOUT CONNECTION. CONNECT TO EXISTING UNDERGROUND DOWNSPOUT CONNECTION.	2/C1
2	PROPOSED DOWNSPOUT. CONNECT TO NEAREST UNDERGROUND STORM MAIN WITH 3-INCH PIPE. LOCATION OF NEAREST STORM MAIN DRAWN IN BASED ON AS-BUILTS. CONTRACTOR TO CONFIRM MAIN LOCATION PRIOR TO DOWNSPOUT INSTALLATION.	3/C1

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REGISTERED PROFESSIONAL ENGINEER
88168
DIGITALLY SIGNED 2025.04.30 15:59:12-07'00'
OREGON
MAY 20, 2014
ANNA LUTTRELL BACKUS
EXPIRATION DATE: 12/31/2025

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CRESWELL HS CANOPY REPLACEMENT

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ISSUE DATE: 5/1/2025

DOWNSPOUT RECONNECTION PLAN

C1

SCALE 1 INCH = 20 FEET
20 0 20 40

File: O:\cip\2025\2500126-CSD-HS-Canopies\CAD\PLOT\2500126-01UT.dwg TAB:C1
Plotted: 4/30/25 at 3:57pm By: ABackus

KEYNOTES

1. DEMOLISH STRUCTURE IN ENTIRETY
2. DEMOLISH CONC PAVING & SUBGRADE AS REQ'D FOR NEW WORK
3. (E) STRUCTURE TO REMAIN
4. (E) CONC STAIRS DOWN TO SUNKEN COURTYARD
5. STEEL CANOPY ASSY
6. LINETYPE INDICATES APPROX. EXTENT/SEPARATION JT & SM COVER, SEE DETAILS
7. 'TEMPORARY' ROOF COVER ASSY, 6"GAP FROM (E) ASSY EA RAKE SIDES - SEE SECTIONS, STRUCTURAL
8. DOWNSPOUT, CONNECT TO (E) STORM SYSTEM, SEE ELEVATIONS, CIVIL
9. PREFIN SM METAL TRIMS & GUTTERS THROUGHOUT WHERE NEW CANOPY OCCURS, SEE DETAILS
10. METAL ROOF DECKING, SEE STRUCTURAL
11. INDICATES SLOPE DIRECTION (DOWN) - APPROX. 1/4":12"
12. (E) STOREFRONT FRAME & GLAZING ASSY W/ WALL ABOVE
13. (E) UTILITIES & PATHWAYS TO REMAIN - WHERE AFFECTED BY DEMOLITION, PROVIDE NEW IN METALLIC RACEWAY/PIPING & RECONNECT TO ORIGINAL FUNCTIONAL CONDITION, ROUTE AT UNDERSIDE/STEEL FRAMING IN NEAT AND CLEAN ORDER, FINISH PAINT TO MATCH CANOPY
14. APPROX. EXTENT/SINGLE PLY MEMBRANE ROOF PATCH W/ RIGID POLYISO CRICKET BELOW, SLOPE TO DRAIN
15. PREFIN SM WALL CAP, SEE SECTION

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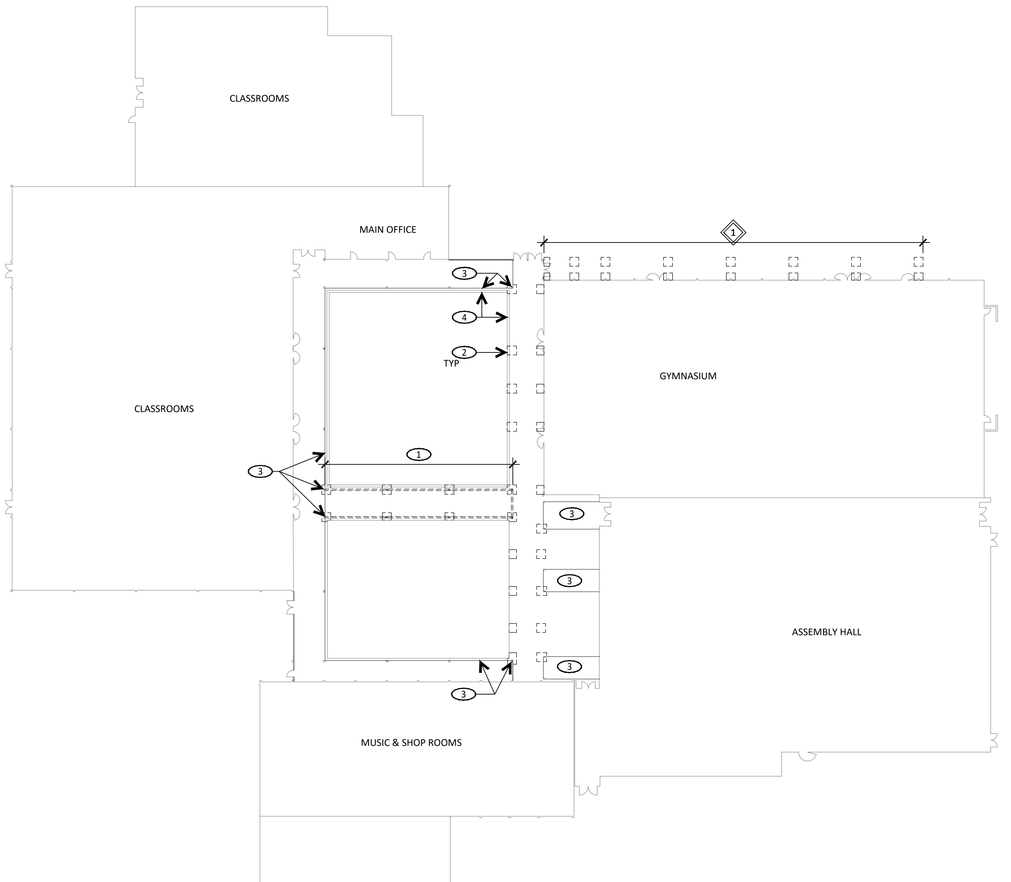
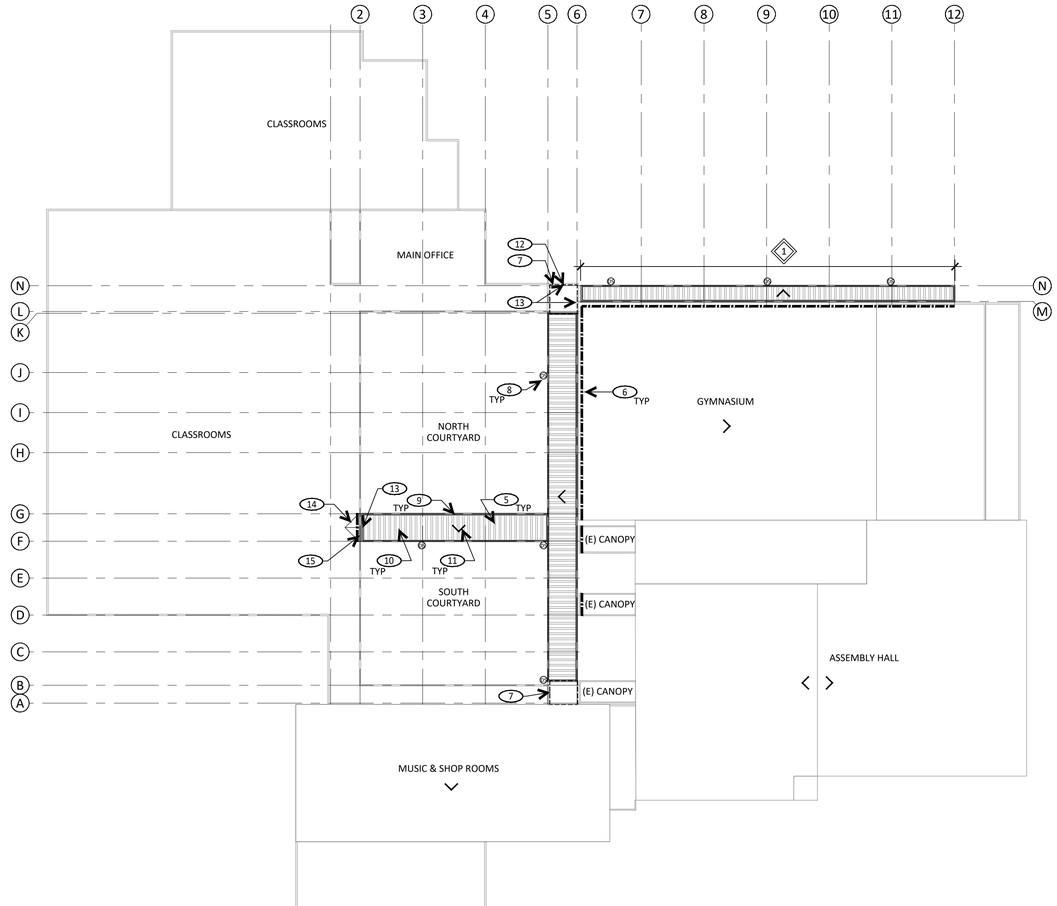


1 DEMOLITION PLAN
1/32" = 1'-0"



2 ROOF PLAN
1/32" = 1'-0"

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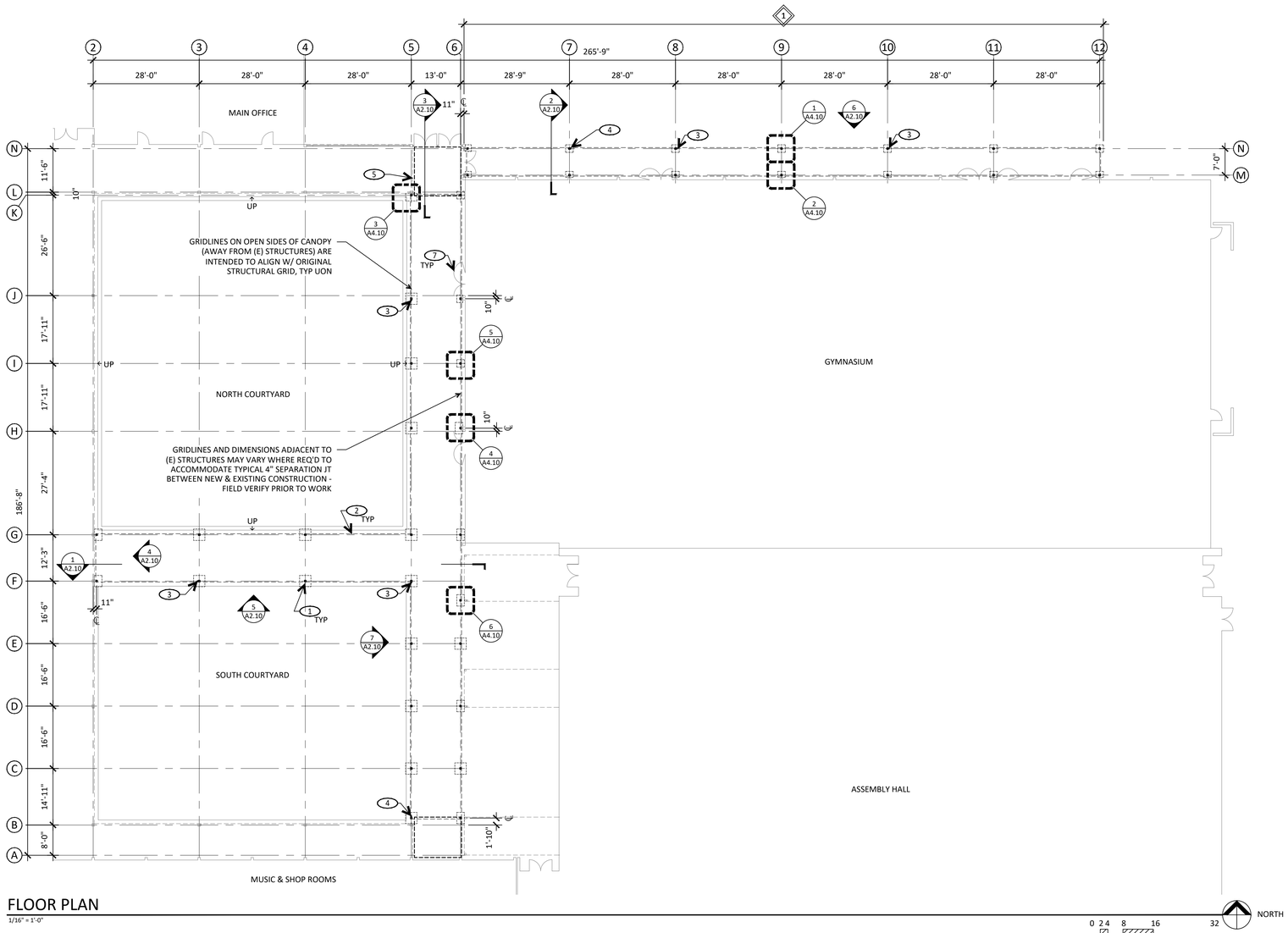
DEMOLITION PLAN & ROOF PLAN

A1.00



1 FLOOR PLAN

1/16" = 1'-0"

NORTH

0 2 4 8 16 32

KEYNOTES

1. CONC FOOTING & HSS COLUMN, SEE STRUCTURAL
2. DASH INDICATES STEEL CANOPY ASSY ABOVE
3. DOWNSPOUT @ (E) LOCATION, CONNECT TO (E) STORM SYSTEM BELOW GRADE, SEE CIVIL
4. DOWNSPOUT CONNECT TO (E) STORM SYSTEM BELOW GRADE, SEE CIVIL
5. TEMPORARY CANOPY ABOVE DASHED IN FOR REFERENCE
6. PATCH CONC. PAVING WHERE DEMOLITION OCCURS, MATCH (E), THICKNESS, DETAILS & EDGE PROFILES, HEAVY BROOM FINISH
7. (E) DOOR, VFY LOCATION TO ENSURE WORK DOES NOT CONFLICT W/ ACCESS

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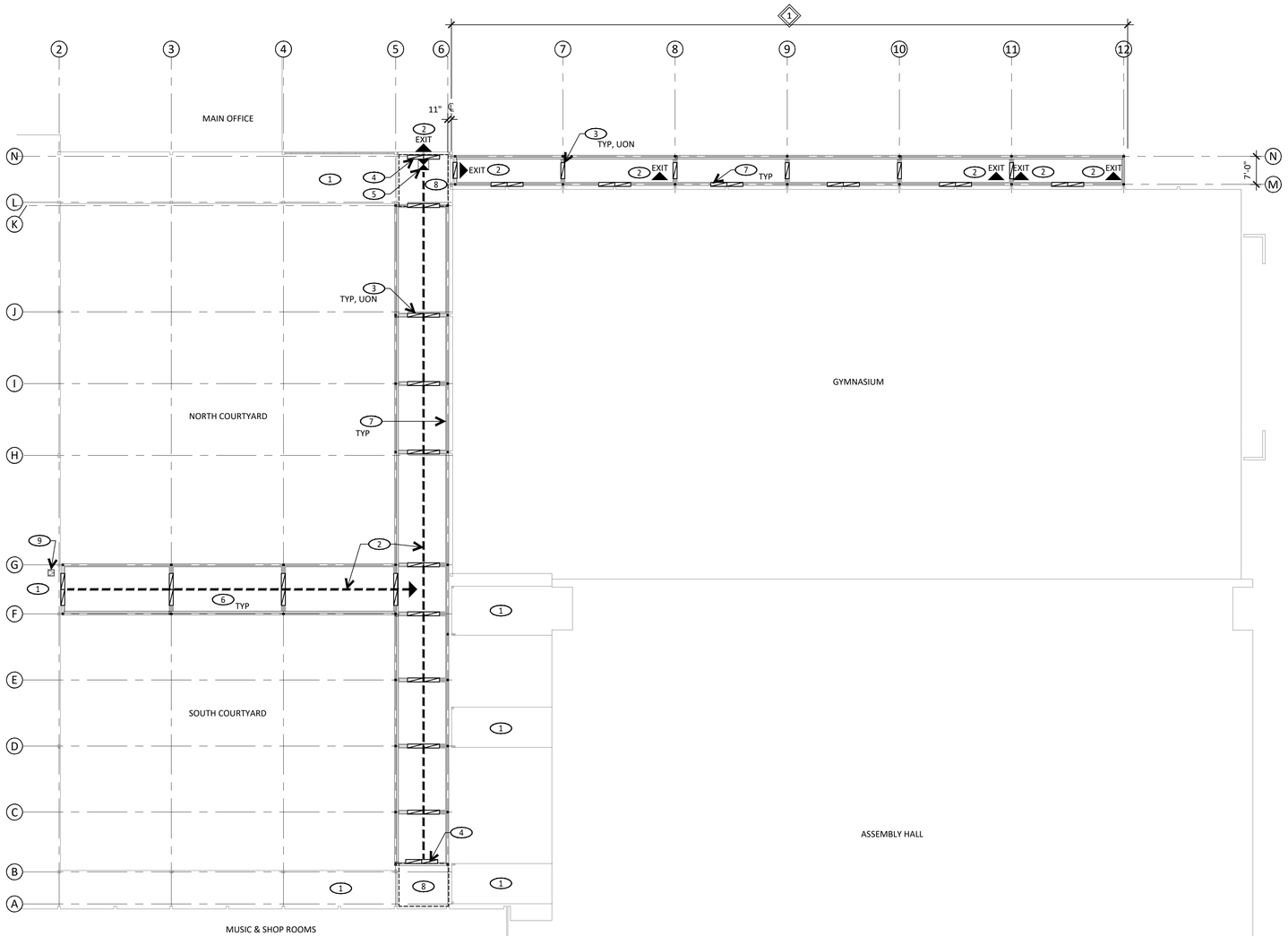
FLOOR PLAN

A1.10



1 ENLARGED REFLECTED CEILING PLAN

1/16" = 1'-0"

0 2 4 8 16 32

NORTH

KEYNOTES

1. (E) CANOPY
2. LINE TYPE & ARROWS INDICATE EMERGENCY EGRESS PATH - PROVIDE AVG 1 FT CANDLE ILLUMINATION AT WALKING SURFACE, CONNECT TO (E) EMERGENCY EGRESS LIGHTING POWER AS REQ'D
3. 8' LINEAR LED LIGHT FIXTURE @ UNDERSIDE/BEAM, CONNECT TO (E) EXTERIOR LIGHTING CIRCUITS & CONTROLS
4. MOUNT FIXTURE TO WALL
5. (E) EXIT SIGN
6. FINISH PAINT SYSTEM UNDERSIDE OF THE STEEL DECKING IN ENTIRETY, MATCH STEEL FRAME
7. ROUTE EXPOSED CONDUITS & PIPING IN NEAT, ORDERLY MANNER AT BEAM BELOW ROOF - DO NOT ROUTE IN ROOF DECK OR FASTEN TO ROOF DECK
8. TEMPORARY ROOF ASSY
9. 16X16 SM ACCESS DOOR @ (E) SOFFIT, FRAME OPENING W/ 2X WD JOISTS AS REQ'D

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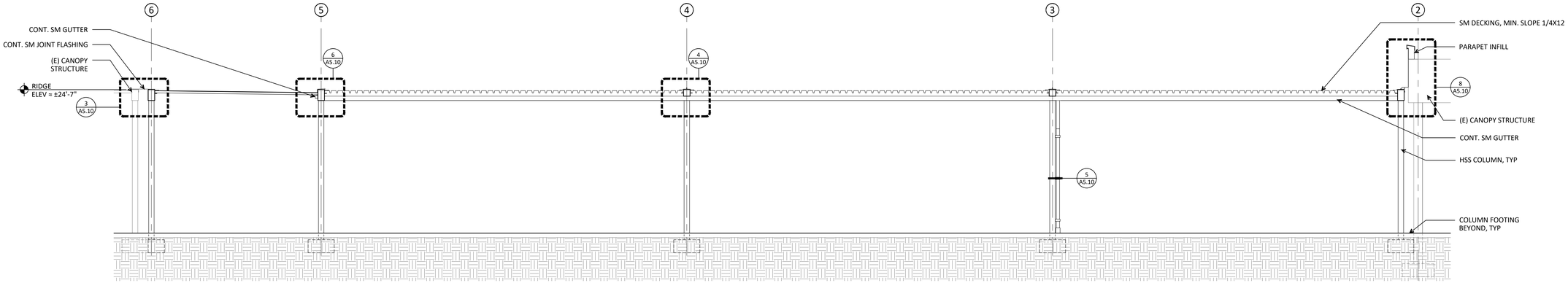
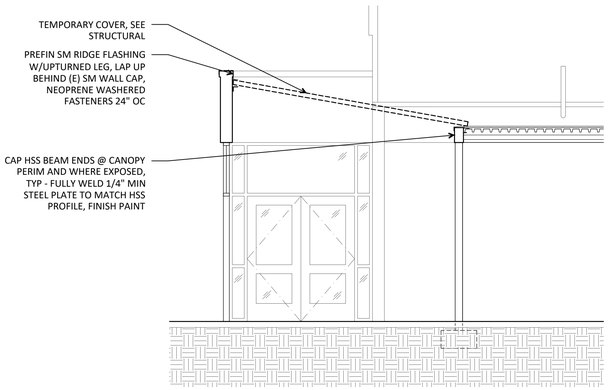
REFLECTED CEILING PLAN

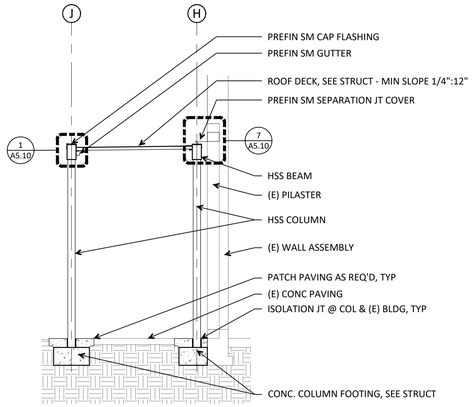
A1.30



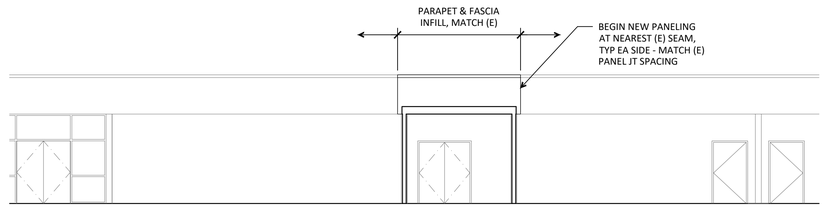
1 CANOPY LONGTITUTE SECTION
1/16" = 1'-0"



2 TEMPORARY ENTRY CANOPY SECTION
1/4" = 1'-0"



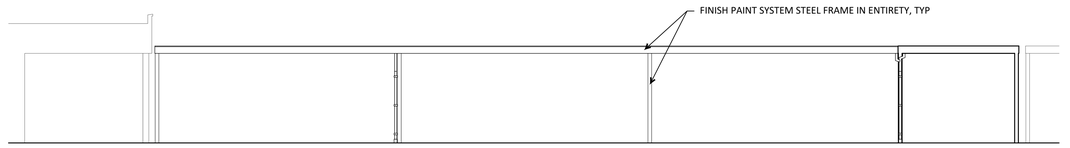
3 CANOPY CROSS SECTION
1/4" = 1'-0"



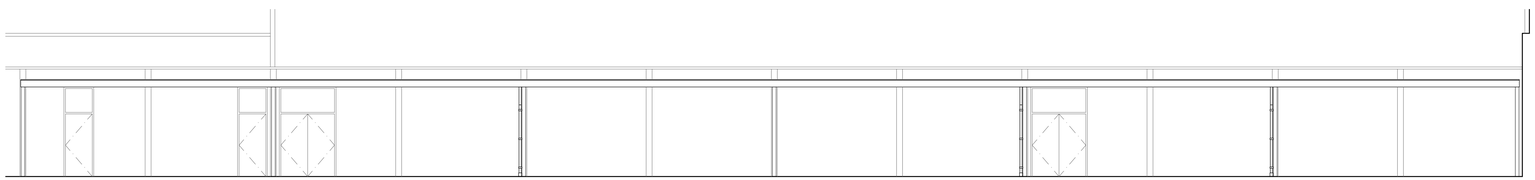
4 ELEVATION - WEST
1/8" = 1'-0"

0 1 2 4 8

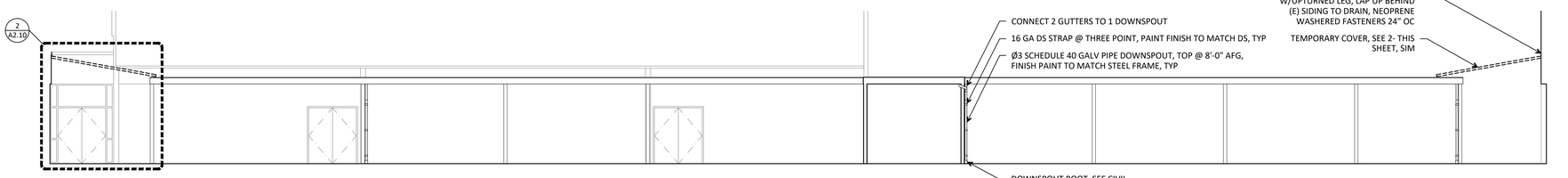
0 2 4 8 16



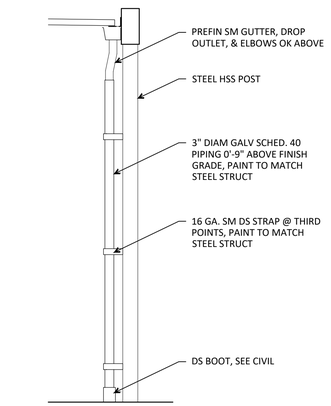
5 ELEVATION - SOUTH
1/8" = 1'-0"



6 ELEVATION - NORTH
1/8" = 1'-0"



8 ELEVATION - EAST
1/8" = 1'-0"



7 TYPICAL DOWNSPOUT
1/8" = 1'-0"

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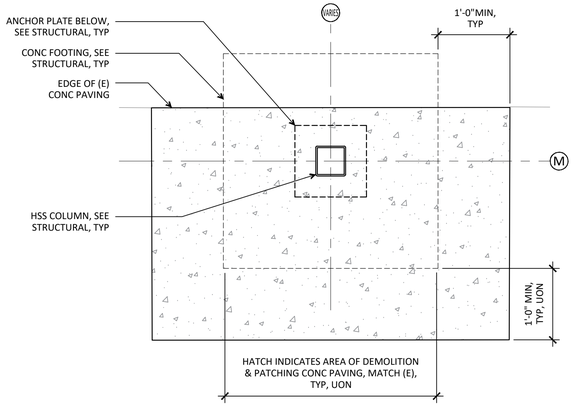
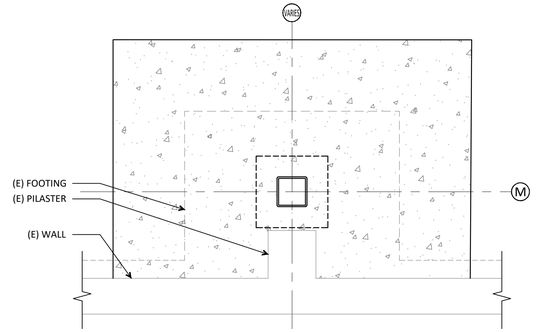
SECTIONS & ELEVATIONS

A2.10

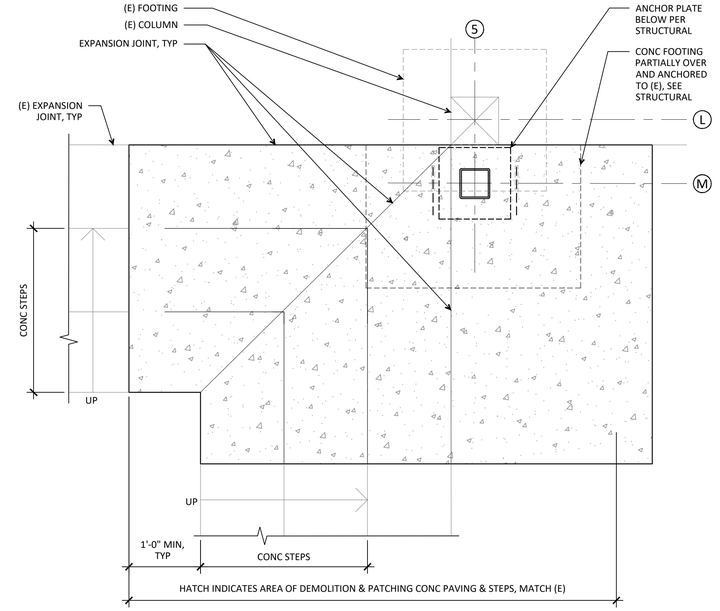


1 TYPICAL COLUMN ENLARGED PLAN
1" = 1'-0"

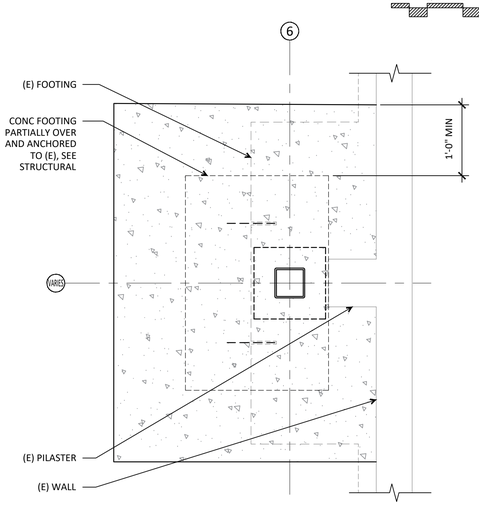
0 1/2 1 2



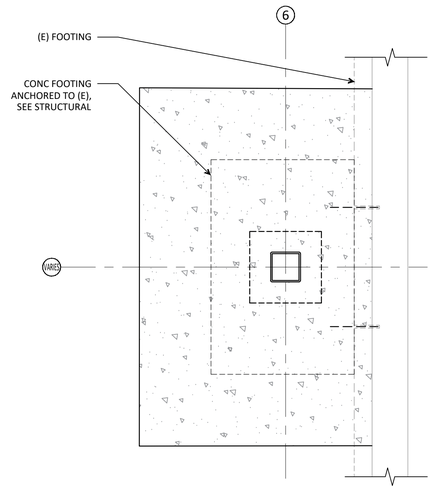
2 TYPICAL COLUMN @ NORTH PILASTER ENLARGED PLAN
1" = 1'-0"



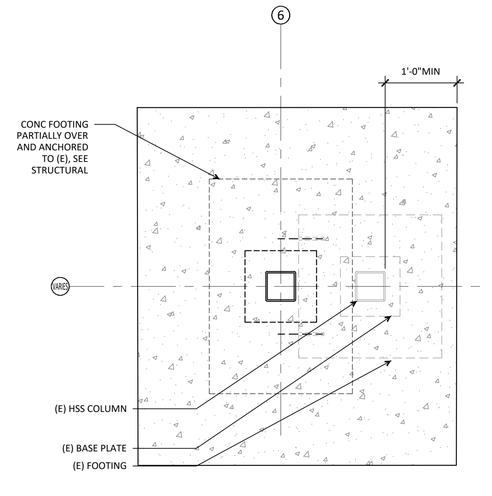
3 TYPICAL COLUMN @ (E) COLUMN ENLARGED PLAN
1" = 1'-0"



4 COLUMN @ WEST PILASTER ENLARGED PLAN
1" = 1'-0"



5 COLUMN @ WEST WALL ENLARGED PLAN
1" = 1'-0"



6 COLUMN @ (E) HSS COLUMN ENLARGED PLAN
1" = 1'-0"

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REVISIONS

CRESWELL HS CANOPY REPLACEMENT

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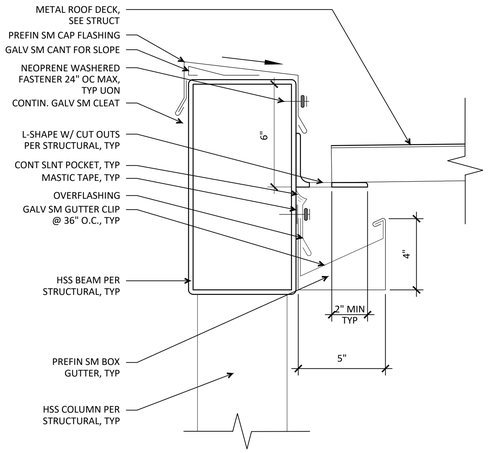
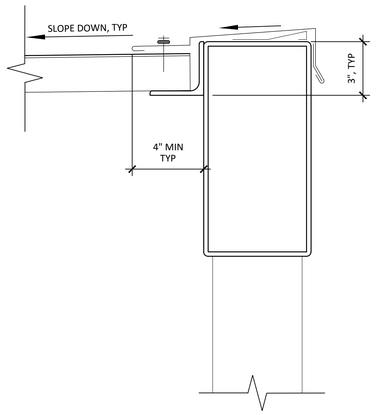
ENLARGED PLANS

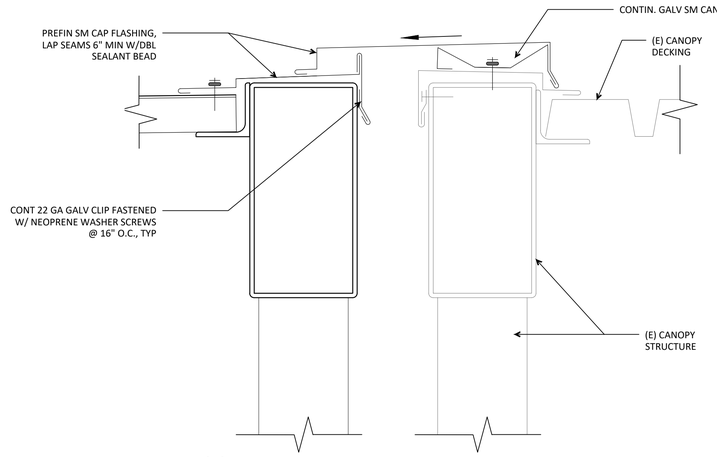
A4.10



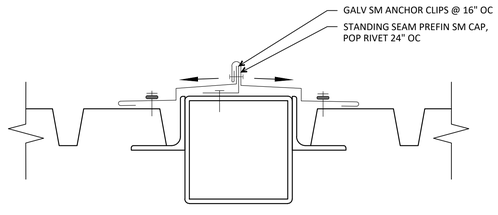
1 TYPICAL EAVE DETAIL
3" = 1'-0"



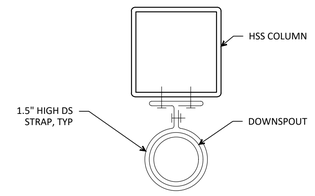
2 TYPICAL RIDGE DETAIL
3" = 1'-0"



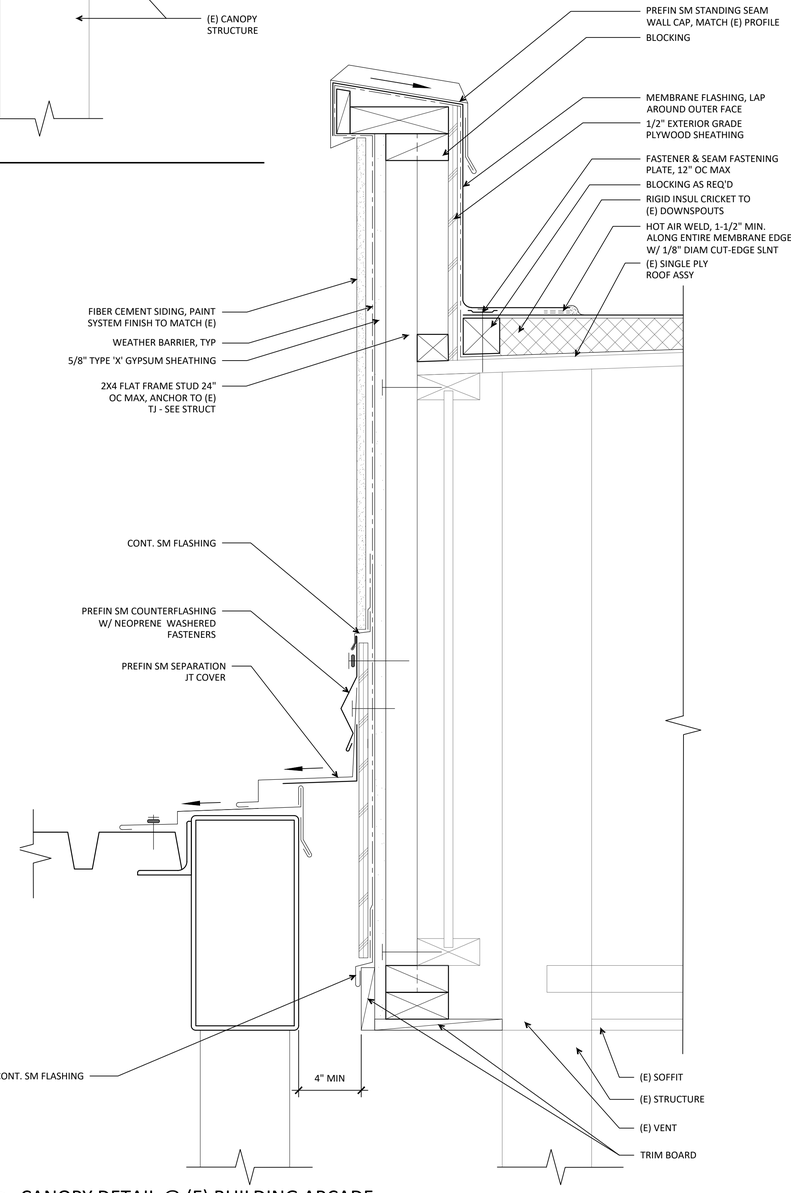
3 JOINT @ (E) CANOPY DETAIL
3" = 1'-0"



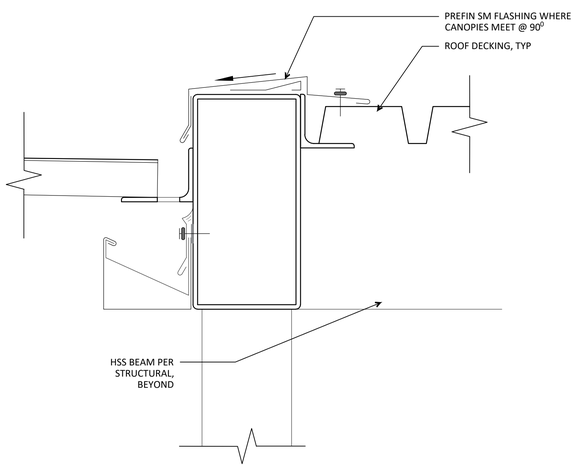
4 TYPICAL CROSS BEAM FLASHING DETAIL
3" = 1'-0"



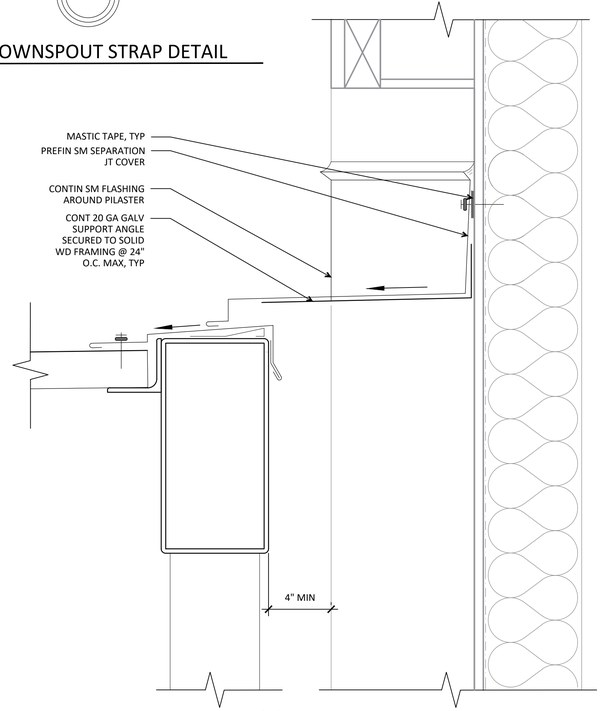
5 TYPICAL DOWNSPOUT STRAP DETAIL
3" = 1'-0"



8 CANOPY DETAIL @ (E) BUILDING ARCADE
3" = 1'-0"



6 CANOPY CROSSING DETAIL
3" = 1'-0"



7 TYPICAL CANOPY DETAIL @ (E) WALL
3" = 1'-0"

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DETAILS

A5.10

DRAWING INDEX

		ISSUE LOG
		CONSTRUCTION DOCUMENTS
S0.01	DRAWING INDEX AND LIST OF ABBREVIATIONS	X
S0.02	GENERAL STRUCTURAL NOTES	X
S0.03	GENERAL STRUCTURAL NOTES CONT.	X
S0.04	SPECIAL INSPECTION	X
S1.10	FOUNDATION PLAN	X
S1.11	CANOPY FRAMING PLAN	X
S5.01	CONCRETE DETAILS	X
S6.01	STEEL DETAILS	X

ISSUE LOG KEY:
' X ' ISSUED AS PART OF A SET
' - ' NOT A PART OF ISSUED SET
' * ' FOR INFORMATION ONLY

DATE 05/01/2025

LIST OF ABBREVIATIONS

A.B.	ANCHOR BOLT
ACI	AMERICAN CONCRETE INSTITUTE
ADD'L.	ADDITIONAL
AESS	ARCHITECTURALLY EXPOSED STRUCTURAL STEEL
AISC	AMERICAN INSTITUTE OF STEEL CONSTRUCTION
ALT.	ALTERNATE
ALUM.	ALUMINUM
ARCH.	ARCHITECT / ARCHITECTURAL
ASCE	AMERICAN SOCIETY OF CIVIL ENGINEERS
ASD	ALLOWABLE STRENGTH DESIGN LOAD LEVEL
ASTM	AMERICAN SOCIETY FOR TESTING AND MATERIALS
AWS	AMERICAN WELDING SOCIETY
BLDG.	BUILDING
BOT.	BOTTOM
BRBF	BUCKLING RESTRAINED BRACED FRAME
C.G.	CENTER OF GRAVITY
C.I.P.	CAST IN PLACE
C.J.	CONTROL JOINT
C.J.P.	COMPLETE JOINT PENETRATION
CL	CENTERLINE
CLR.	CLEAR
CLT	CROSS LAMINATED TIMBER
CMU	CONCRETE MASONRY UNIT
COL.	COLUMN
CONC.	CONCRETE
CONN.	CONNECTION
CONST.	CONSTRUCTION
CONT.	CONTINUOUS
db	BAR DIAMETER
DBA	DEFORMED BAR ANCHOR
DET.	DETAIL
DIA., Ø	DIAMETER
DIAG.	DIAGONAL
D.L.	DEAD LOAD
DLT	DOWEL LAMINATED TIMBER
DWG.	DRAWING
ELEC.	ELECTRICAL
EL.	ELEVATION
EQ.	EQUAL
EXIST., (E)	EXISTING
EXP.	EXPANSION
EXT.	EXTERIOR
FDN.	FOUNDATION
FIN.	FINISH
FLR.	FLOOR
FRT	FIRE RETARDANT TREATED
FT.	FOOT
FTG.	FOOTING
GA.	GAUGE
GALV.	GALVANIZED
GL	GLULAM
GL	GLULAM
HORIZ.	HORIZONTAL
HSS	HOLLOW STRUCTURAL STEEL
IBC	INTERNATIONAL BUILDING CODE
I.D.	INSIDE DIAMETER
IN.	INCHES
INT.	INTERIOR
K	KIPS
KSF	KIPS PER SQUARE FOOT
KSI	KIPS PER SQUARE INCH
LBS.	POUNDS
L.L.	LIVE LOAD
LLH	LONG LEG HORIZONTAL
LLV	LONG LEG VERTICAL
LOC.	LOCATION
LONG.	LONGITUDINAL
LSL	LAMINATED STRAND LUMBER
LVF	LOW VELOCITY FASTENER
LVL	LAMINATED VENEER LUMBER
MAX.	MAXIMUM
MBMA	METAL BUILDING MANUFACTURERS ASSOCIATION
MECH.	MECHANICAL
MEPF	MECHANICAL, ELECTRICAL, PLUMBING AND FIRE SAFETY
MFR.	MANUFACTURER
MIN.	MINIMUM
MISC.	MISCELLANEOUS
MPH	MILES PER HOUR
MPP	MASS PLYWOOD PANELS
MT	MAGNETIC PARTICLE TESTING
(N)	NEW
N.I.C.	NOT IN CONTRACT
NLT	NAIL LAMINATED TIMBER
NOM.	NOMINAL
NO.	NUMBER
N.T.S.	NOT TO SCALE
o.c.	ON CENTER
O.D.	OUTSIDE DIAMETER
OPP.	OPPOSITE
OSL	ORIENTED STRAND LUMBER
OWJ	OPEN WEB JOIST
PAF	POWDER ACTUATED FASTENER
PART.	PARTITION
P/C	PRECAST
PCF	POUNDS PER CUBIC FOOT
PERIM.	PERIMETER
PL	PLATE
PP	PARTIAL PENETRATION
PSF	POUNDS PER SQUARE FOOT
PSL	PARALLEL STRAND LUMBER
PSI	POUNDS PER SQUARE INCH
P/T	POST-TENSIONED
P.T.	PRESSURE TREATED
PVC	POLYVINYL CHLORIDE
R, RAD.	RADIUS
RCSC	RESEARCH COUNCIL ON STRUCTURAL CONNECTIONS
REF.	REFERENCE
RET.	RETURN
REINF.	REINFORCING
REQ'D.	REQUIRED
REQ'MTS.	REQUIREMENTS
SCHED.	SCHEDULE
S.C.	SLIP CRITICAL
SCL	STRUCTURAL COMPOSITE LUMBER
SIM.	SIMILAR
SLFS	SEISMIC FORCE RESISTING SYSTEM
S.O.G.	SLAB ON GRADE
SPEC.	SPECIFICATION
SQ.	SQUARE
SS	STAINLESS STEEL
SSMA	STEEL STUD MANUFACTURERS ASSOCIATION
STD.	STANDARD
STRUCT.	STRUCTURAL
SYM.	SYMMETRICAL
THRU	THROUGH
T&G	TONGUE AND GROOVE
TRANS.	TRANSVERSE
TS	LIGHT GAUGE TUBE STEEL
TYP.	TYPICAL
U.N.O.	UNLESS NOTED OTHERWISE
U.T.	ULTRASONIC TESTING
ULT.	ULTIMATE STRENGTH DESIGN LOAD LEVEL
VERT.	VERTICAL
V.I.F.	VERIFY IN FIELD
w/	WITH
WF	WIDE FLANGE
w/o	WITHOUT
W.P.	WORK POINT
WPS	WELDING PROCEDURE SPECIFICATION
WWF	WELDED WIRE FABRIC

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DRAWING INDEX AND LIST OF ABBREVIATIONS

S0.01

GENERAL STRUCTURAL NOTES

GENERAL

STRUCTURAL DRAWINGS ARE A PART OF THE CONTRACT DOCUMENTS AND ARE COMPLEMENTARY TO THE ARCHITECTURAL, MECHANICAL, ELECTRICAL, PLUMBING DRAWINGS, THE SPECIFICATIONS AND OTHER CONTRACT DOCUMENTS. THE CONTRACTOR IS RESPONSIBLE FOR COORDINATING THE REQUIREMENTS FROM THE CONTRACT DOCUMENTS INTO THEIR SHOP DRAWINGS AND WORK. AS REQUIRED BY THE GENERAL CONDITIONS, THE CONTRACTOR SHALL PROMPTLY REPORT TO THE ARCHITECT ANY ERRORS, INCONSISTENCIES, OR OMISSIONS IN THE CONTRACT DOCUMENTS DISCOVERED BY OR MADE KNOWN TO THE CONTRACTOR.

THE GENERAL STRUCTURAL NOTES SUPPLEMENT THE PROJECT SPECIFICATIONS. REFER TO THE PROJECT SPECIFICATIONS FOR ADDITIONAL REQUIREMENTS. NOTES AND DETAILS ON THE STRUCTURAL DRAWINGS SHALL TAKE PRECEDENCE OVER THE GENERAL STRUCTURAL NOTES AND TYPICAL DETAILS. WHERE NO DETAILS ARE GIVEN, CONSTRUCTION SHALL BE AS SHOWN FOR SIMILAR WORK. WHERE CONFLICT EXISTS, THE MORE STRINGENT OR RESTRICTIVE REQUIREMENT SHALL GOVERN UNTIL CLARIFICATION IS REQUESTED.

CODE REQUIREMENTS:
CONFORM TO THE 2022 OREGON STRUCTURAL SPECIALTY CODE (OSSC), BASED ON THE 2021 INTERNATIONAL BUILDING CODE (IBC).

TEMPORARY CONDITIONS:
THE STRUCTURE IS DESIGNED TO FUNCTION AS A UNIT UPON COMPLETION. THE CONTRACTOR IS RESPONSIBLE FOR FURNISHING ALL TEMPORARY BRACING AND/OR SUPPORT THAT MAY BE REQUIRED AS THE RESULT OF THE CONTRACTOR'S CONSTRUCTION METHODS AND/OR SEQUENCES UNTIL COMPLETION.

CONTRACTOR'S CONSTRUCTION AND/OR ERECTION SEQUENCES SHALL RECOGNIZE AND CONSIDER THE EFFECTS OF THERMAL MOVEMENTS OF STRUCTURAL ELEMENTS DURING THE CONSTRUCTION PERIOD.

EXCAVATIONS SHALL NOT REDUCE THE VERTICAL OR LATERAL SUPPORT FOR ANY FOUNDATION OF THIS PROJECT OR ANY ADJACENT STRUCTURE WITHOUT FIRST UNDERPINNING OR PROTECTING THE FOUNDATION AGAINST DETRIMENTAL LATERAL AND/OR VERTICAL MOVEMENT.

EXISTING CONDITIONS:
ALL EXISTING CONDITIONS, DIMENSIONS AND ELEVATIONS SHALL BE FIELD VERIFIED. THE CONTRACTOR SHALL NOTIFY THE ARCHITECT OF ANY SIGNIFICANT DISCREPANCIES FROM CONDITIONS SHOWN ON THE DRAWINGS.

ASSUMED FUTURE CONSTRUCTION:
VERTICAL: NONE
HORIZONTAL: SECOND PHASE OF CANOPY REPLACEMENT IS EXPECTED

DESIGN CRITERIA

DESIGN WAS BASED ON THE STRENGTH AND DEFLECTION CRITERIA OF THE OSSC. IN ADDITION TO THE DEAD LOADS, THE FOLLOWING LOADS AND ALLOWABLES WERE USED FOR DESIGN, WITH LIVE LOADS (L.L.) REDUCED PER OSSC:

GRAVITY SYSTEM CRITERIA		
OCCUPANCY OR USE	**UNIFORM LOAD**	**CONCENTRATED LOAD**
ROOF LIVE/SNOW LOAD	27 PSF SNOW LOAD (ALSO SEE SNOW LOAD CRITERIA BELOW)	
GRAVITY LOADING NOTES:	1. LIVE LOADS REDUCED PER OSSC. 2. MEMBERS DESIGNED FOR MORE CRITICAL OF UNIFORM OR CONCENTRATED LOAD.	
FRAMING MEMBER	**DEFLECTION LIMITS**	
VERTICAL ROOF FRAMING DEFLECTION - BUILDING INTERIOR	l/240 DUE TO SNOW LOAD AND l/180 DUE TO SNOW LOAD PLUS LONG TERM CREEP COMPONENT OF D.L.	
DEFLECTION LIMIT NOTES:	1. l = LENGTH OF STRUCTURAL FRAMING MEMBER BETWEEN SUPPORTS. 2. FOR CANTILEVER FRAMING, l SHALL BE TAKEN AS 2X THE CANTILEVER LENGTH. 3. LONG TERM CREEP COMPONENT OF DEAD LOAD (D.L.) INCLUDES ONLY THE TIME DEPENDENT PORTION OF LONG TERM DEAD LOADING AND IS APPLICABLE TO CONCRETE AND WOOD FRAMING ONLY.	
SNOW CRITERIA		
DESIGN ROOF SNOW LOAD	27 PSF MINIMUM IN ACCORDANCE WITH OSSC	
SNOW DRIFT	PER OSSC AS SHOWN ON PLANS	
GROUND SNOW LOAD	Pg= 14 PSF IN ACCORDANCE WITH: snowload.seao.org	
FLAT ROOF SNOW LOAD	Pf = 11 PSF	
SNOW EXPOSURE FACTOR	Ce = 1.0	
SNOW LOAD IMPORTANCE FACTOR	Is = 1.1	
THERMAL FACTOR	Ct = 1.0	
GEOTECHNICAL CRITERIA		
DESIGN BASED ON REPORT BY:	PBS ENGINEERING AND ENVIRONMENTAL INC. DATED OCT. 4, 2023	
ALLOWABLE SOIL PRESSURE:		
NATIVE SOIL	2,000 PSF	
SHORT TERM LOADING	1/3 INCREASE	
WIND CRITERIA		
RISK CATEGORY	III	
BASIC WIND SPEED	VULT = 105 MPH (3-SECOND GUST)	VASD = 81 MPH (3-SECOND GUST)
EXPOSURE CATEGORY	B	
GUST / INTERNAL PRESSURE	GCpi = +/- 0.18	
SEISMIC CRITERIA		
RISK CATEGORY	III	
SEISMIC DESIGN CATEGORY	D	
SITE CLASS	D	
SEISMIC IMPORTANCE FACTOR	IE = 1.25	
MAPPED SPECTRAL ACCELERATION PARAMETERS	SS = 0.66	S1 = 0.38
DESIGN SPECTRAL RESPONSE ACCELERATION PARAMETERS	SDS = 0.56	SD1 = 0.40

STRUCTURAL OBSERVATIONS

THE STRUCTURAL ENGINEER OF RECORD (SEOR) WILL PERFORM STRUCTURAL OBSERVATIONS BASED ON THE REQUIREMENTS OF THE OSSC AT THE STAGES OF CONSTRUCTION LISTED BELOW. CONTRACTOR SHALL PROVIDE SUFFICIENT ADVANCED NOTICE AND ACCESS FOR THE SEOR TO PERFORM THESE OBSERVATIONS.

ITEM	COMMENTS
PRIOR TO FIRST CONCRETE POUR	AFTER REBAR PLACEMENT
DURING INITIAL STEEL ERECTION	
AS REQUIRED TO ADDRESS STRUCTURAL ISSUES	

A FIELD REPORT WILL BE SUBMITTED TO THE BUILDING DEPARTMENT FOLLOWING EACH SITE VISIT.

STRUCTURAL OBSERVATION IS FOR THE GENERAL CONFORMANCE OF THE STRUCTURAL DRAWINGS AND DOES NOT ALLEVIATE ANY SPECIAL INSPECTION REQUIREMENTS.

SPECIAL INSPECTIONS AND TESTING

SPECIAL INSPECTION WILL BE PROVIDED BY THE OWNER BASED ON THE REQUIREMENTS OF THE OSSC AS SUMMARIZED IN THE SPECIAL INSPECTION AND TESTING PROGRAM ON SHEETS S0.04. CONTRACTOR SHALL PROVIDE SUFFICIENT NOTICE AND ACCESS FOR THE SPECIAL INSPECTOR TO PERFORM THESE INSPECTIONS.

SUBMITTALS

SUBMIT SHOP DRAWINGS AND OTHER SUBMITTALS TO THE ARCHITECT AND ENGINEER PRIOR TO FABRICATION AND CONSTRUCTION OF STRUCTURAL ITEMS IN ACCORDANCE WITH THE SPECIFICATIONS. IF THE SUBMITTALS DIFFER FROM OR ADD TO THE STRUCTURAL CONTRACT DOCUMENTS, THEY SHALL BEAR THE SEAL AND SIGNATURE OF A STRUCTURAL ENGINEER REGISTERED IN THE STATE OF OREGON. ANY CHANGES TO THE STRUCTURAL DRAWINGS SHALL BE SUBMITTED TO THE ARCHITECT AND ARE SUBJECT TO REVIEW AND ACCEPTANCE BY THE SEOR.

FIELD ENGINEERED DETAILS DEVELOPED BY THE CONTRACTOR THAT DIFFER FROM OR ADD TO THE STRUCTURAL DRAWINGS SHALL BEAR THE SEAL AND SIGNATURE OF A PROFESSIONAL ENGINEER REGISTERED IN THE STATE OF OREGON AND SHALL BE SUBMITTED TO THE ARCHITECT PRIOR TO CONSTRUCTION.

THE USE OF REPRODUCTIONS OR PHOTOCOPIES OF THE CONTRACT DRAWINGS SHALL NOT BE PERMITTED. WHEN CAD OR REVIT FILES ARE PROVIDED TO THE CONTRACTOR OR SUBCONTRACTORS, IT IS THE RESPONSIBILITY OF THE CONTRACTOR/SUBCONTRACTOR TO REMOVE ALL INFORMATION NOT DIRECTLY RELEVANT TO THE SCOPE OF THE SUBMITTAL AS WELL AS ALL REFERENCES TO OUTSIDE SOURCE FILES.

DELEGATED DESIGN SUBMITTALS SHALL INCLUDE DESIGN DRAWINGS AND CALCULATIONS FOR ITEMS THAT ARE DESIGNED BY OTHERS. DELEGATED DESIGN SUBMITTALS SHALL BEAR THE SEAL AND SIGNATURE OF A PROFESSIONAL ENGINEER REGISTERED IN THE STATE OF OREGON ON EVERY DRAWING SHEET AND ON THE CALCULATION COVER SHEET, AND SHALL BE SUBMITTED TO THE ARCHITECT AND ENGINEER PRIOR TO FABRICATION. CALCULATIONS AND DETAILS SHALL BE INCLUDED FOR ALL CONNECTIONS TO THE STRUCTURE, CONSIDERING LOCALIZED EFFECTS ON STRUCTURAL ELEMENTS. DESIGN SHALL BE BASED ON THE REQUIREMENTS OF THE OSSC AND AS NOTED UNDER "DESIGN CRITERIA".

SUBMITTALS AND DELEGATED DESIGN SUBMITTALS SHALL INCLUDE THE FOLLOWING:

ITEM	SUBMITTAL	DELEGATED DESIGN SUBMITTAL	COMMENTS
CONCRETE MIX DESIGNS	X		
CONCRETE REINFORCEMENT	X		
CONCRETE ANCHORAGES	X		
EMBEDDED STEEL ITEMS	X		
STRUCTURAL STEEL	X		
STEEL WELDING PROCEDURES	X		
STEEL JOISTS AND JOIST GIRDERS		X	
STEEL DECKING	X		

TABLE NOTES:

1. CONTRACTOR SHALL COORDINATE AND SHOW ALL REQUIRED PENETRATIONS, WITH DIMENSIONS FOR MECHANICAL, ELECTRICAL, PLUMBING, FIRE PROTECTION, TECHNOLOGY AND OTHER SERVICES ON A SINGLE DRAWING FOR REVIEW AT EACH SLAB/DECK, STRUCTURAL WALL AND/OR BEAM.

CONCRETE MIX DESIGNS

CONCRETE WORK SHALL CONFORM TO CHAPTER 19 OF THE OSSC. CONCRETE STRENGTHS SHALL BE VERIFIED BY STANDARD CYLINDER TESTS PER ASTM C39. CONCRETE MIX TO BE DESIGNED AND PROPORTIONED BY THE CONTRACTOR IN ACCORDANCE WITH ACI 318-19 CHAPTER 26, ACI 301-16 SECTION 4 AND THE FOLLOWING INFORMATION:

MIX TYPE	USE	f'c (PSI)	TEST AGE (DAYS)	MAX. W/CM RATIO	MAX. AGG. SIZE	EXPOSURE CLASS			
A	SPREAD FOOTINGS	4,000	28	N/A	1"	F0	S0	W0	C0

TABLE NOTES:

1. REF. ACI 318-19 TABLE 19.3.2.1 FOR ADDITIONAL MIX REQUIREMENTS SPECIFIC TO EXPOSURE CLASS.
2. ALL CONCRETE MIXES TO BE NORMAL WEIGHT CONCRETE, U.N.O.
3. EXPOSURE CATEGORY "F" APPLIES TO LEVEL OF FREEZE THAW EXPOSURE.
4. EXPOSURE CATEGORY "S" APPLIES TO LEVEL OF SULFATE EXPOSURE.
5. EXPOSURE CATEGORY "W" APPLIES TO REQUIRED LEVEL OF PERMEABILITY.
6. EXPOSURE CATEGORY "C" APPLIES TO CORROSIVE LOCATIONS - INCLUDING SURROUNDING ENVIRONMENT (SUCH AS MARINE ENVIRONMENT) AND CORROSIVE SOILS.
7. ESTABLISH WATER-CEMENTITIOUS MATERIAL RATIO PER ACI 301-16 SECTION 4.
8. VERIFY WATER-CEMENTITIOUS MATERIAL RATIO WITH FLOOR COVERING MANUFACTURER FOR CONCRETE FLOORS WITH MOISTURE SENSITIVE FLOOR COVERINGS.

PORTLAND CEMENT CONTENT MAY BE REPLACED WITH FLY ASH CONFORMING TO ASTM C618 (INCLUDING TABLE 2A) TYPE F OR TYPE C, SLAG CEMENT CONFORMING TO ASTM C989, AND SILICA FUME CONFORMING TO ASTM C1240 PROVIDED THAT THE MIX STRENGTH IS SUBSTANTIATED BY TEST DATA.

FOR MIX DESIGNS WITH f'c = 5,000 PSI OR LESS, SLAG CEMENT MAY BE SUBSTITUTED FOR FLY ASH AT A 1:1 RATIO WITHOUT TEST DATA. WHEN SLAG CEMENT IS SUBSTITUTED IN HIGHER STRENGTH MIXES OR AT A DIFFERENT RATIO, THE MIX STRENGTH MUST BE SUBSTANTIATED BY TEST DATA.

ALL CONCRETE SUBJECT TO EXPOSURE CLASSES F1, F2 OR F3 SHALL BE AIR ENTRAINED. AIR-ENTRAINING AGENTS SHALL CONFORM TO ASTM C260. THE AMOUNT OF ENTRAINED AIR SHALL BE ACCORDING TO ACI 318-19 TABLE 19.3.3.1 AS INDICATED BELOW WITH A FIELD TOLERANCE OF ± 1.5 PERCENT BY VOLUME. THE AMOUNT OF ENTRAINED AIR SHALL BE MEASURED IN THE FIELD AT THE DISCHARGE FROM THE TRUCK.

CONCRETE MIX AIR CONTENT		
MAX. AGGREGATE SIZE	**CONCRETE SUBJECT TO FREEZE/THAW (EXPOSURE CLASS F1)**	**CONCRETE SUBJECT TO CONT. MOISTURE AND/OR DEICING CHEMICALS (EXPOSURE CLASS F2 AND F3)**
3/8"	6.0%	7.5%
1/2"	5.5%	7.0%
3/4"	5.0%	6.0%
1"	4.5%	6.0%
1-1/2"	4.5%	5.5%
ANY WET-MIX SHOTCRETE	5.0%	6.0%

A WATER-REDUCING ADMIXTURE CONFORMING TO ASTM C494 USED IN STRICT ACCORDANCE WITH THE MANUFACTURER'S RECOMMENDATIONS SHALL BE INCORPORATED IN CONCRETE MIX DESIGNS. A HIGH-RANGE WATER-REDUCING (HRWR) ADMIXTURE CONFORMING TO ASTM C494 TYPE F OR G MAY BE USED IN CONCRETE MIXES PROVIDING THAT THE SLUMP DOES NOT EXCEED 10".

THE CONTRACTOR SHALL SUBMIT CONCRETE MIX DESIGNS AND TEST DATA COMPLIANT WITH ACI 301-16 AND ACI 318-19 A MINIMUM OF TWO WEEKS PRIOR TO PLACING CONCRETE. NO WATER MAY BE ADDED TO CONCRETE IN THE FIELD UNLESS SPECIFICALLY APPROVED IN WRITING BY THE CONCRETE SUPPLIER AND SEOR IN CONJUNCTION WITH THE CONCRETE MIX DESIGN.

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GENERAL STRUCTURAL NOTES

S0.02

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GENERAL STRUCTURAL NOTES CONT.

CONCRETE REINFORCING STEEL

CONCRETE REINFORCEMENT SHALL BE AS LISTED BELOW. ASTM A615 REINFORCEMENT MAY BE SUBSTITUTED FOR ASTM A706 REINFORCEMENT PROVIDED THAT THE ACTUAL YIELD STRENGTH BASED ON MILL TESTS DOES NOT EXCEED Fy BY MORE THAN 18,000 PSI AND THE RATIO OF ACTUAL TENSILE STRENGTH TO ACTUAL YIELD STRENGTH IS NOT LESS THAN 1.25 AND THE ELONGATION REQUIREMENTS OF ASTM A706 ARE MET PER ACI 318-19 SECTION 20.2.2.5. MILL TESTS CERTIFICATIONS FOR SUBSTITUTED BARS SHALL BE SUBMITTED TO THE SPECIAL INSPECTOR AND SEOR PRIOR TO PLACEMENT. ASTM A706 REINFORCEMENT MAY BE SUBSTITUTED FOR ASTM A615 REINFORCEMENT.

REINFORCING LOCATION	MATERIAL GRADE
REINFORCING TO BE WELDED	ASTM A706 GRADE 60
ALL OTHER USES U.N.O.	ASTM A615 GRADE 60

ALL REINFORCING STEEL SHALL BE SECURELY TIED IN PLACE WITH #16 ANNEALED IRON WIRE. BARS IN BEAMS AND SLABS SHALL BE SUPPORTED ON WELL-CURED CONCRETE BLOCKS OR APPROVED METAL OR PLASTIC CHAIRS, AS SPECIFIED BY THE CRSI MANUAL OF STANDARD PRACTICE, MSP-1. REINFORCING STEEL SHALL BE DETAILED IN ACCORDANCE WITH ACI MNL-66 "ACI DETAILING MANUAL". SHOP DRAWINGS SHALL INCLUDE ELEVATIONS OF ALL BEAMS, WALLS AND COLUMNS SHOWING BAR LOCATIONS.

REINFORCING BARS SHALL NOT BE BENT OR STRAIGHTENED IN THE FIELD WITHOUT APPROVAL OF THE SEOR. PREHEATING METHODS SHALL BE SUBMITTED TO THE SEOR FOR APPROVAL PRIOR TO BENDING OF BARS #6 OR LARGER.

LAP ALL REINFORCING BARS PER THE TYPICAL LAP SPLICE LENGTH SCHEDULES, EXCEPT AS NOTED ON DRAWINGS. USE LAP LENGTH FOR SMALLER BAR WHEN SPLICING DIFFERENT BAR SIZES. BARS SPLICED WITH NONCONTACT LAPS SHALL BE SPACED NO FARTHER THAN 1/5TH THE LAP LENGTH OR 6 INCHES. MECHANICAL SPLICES NOTED ON THE PLANS SHALL BE DAYTON SUPERIOR BAR-LOCK OR TAPER-LOCK COUPLERS (UES ER-319) OR APPROVED EQUAL WITH A CURRENT EVALUATION REPORT. FOR CONCRETE STRENGTHS BETWEEN TABULATED VALUES, USE LAP SPLICE LENGTHS FOR NEAREST LOWER CONCRETE STRENGTH INDICATED.

TYP. FOUNDATION LAP SPLICE LENGTH SCHEDULE (IN.) - 60 KSI		
BAR SIZE	BOTTOM BARS	TOP BARS
	4,000 PSI	4,000 PSI
#3	14	20
#4	20	26
#5	24	32
#6	28	38
#7	42	54

TABLE NOTES:

1. SPLICE LENGTHS ARE FOR MEMBERS THAT ARE NOT PART OF THE SFRS. REFERENCE PLANS AND ELEVATIONS FOR LAP SPLICES LENGTHS AT SFRS ELEMENTS.
2. SPLICE LENGTHS APPLY TO ASTM A615 OR ASTM A706 GRADE 60 DEFORMED REINFORCING BARS ONLY.
3. SPLICE LENGTHS ARE BASED ON THE CLEAR COVER AND MINIMUM BAR CLEAR SPACING INDICATED BELOW.
4. SPLICE LENGTHS APPLY TO NORMAL WEIGHT CONCRETE ONLY. MULTIPLY TABLE VALUES BY 1.3 FOR LIGHT-WEIGHT CONCRETE.
5. SPLICE LENGTHS APPLY TO UNCOATED BARS ONLY. MULTIPLY TABLE VALUES BY 1.5 FOR EPOXY-COATED BARS.
6. COMBINATIONS OF EFFECTS DUE TO CONCRETE STRENGTH, CONCRETE WEIGHT, AND EPOXY COATING ARE CUMULATIVE.
7. SLAB, FOUNDATION AND MAT TOP BARS ARE BARS CAST ABOVE MORE THAN 12" OF FRESH CONCRETE. ALL OTHER SLAB BARS MAY BE CONSIDERED BOTTOM BARS.

REINFORCING STEEL SHALL HAVE PROTECTION AND SPACING AS FOLLOWS:

CONCRETE COVER		
USE	CLEAR COVER	MIN. CLEAR SPACING
CONCRETE EXPOSED TO EARTH OR WEATHER	1-1/2" (#5 AND SMALLER) 2" (#6 AND LARGER)	2db OR 1"
CONCRETE CAST AGAINST AND EXPOSED TO EARTH	3"	3db OR 1"

POST-INSTALLED CONCRETE ANCHORS

POST-INSTALLED CONCRETE ANCHORS SHALL BE THE FOLLOWING PRODUCTS, U.N.O.:

TYPE	APPROVED ANCHOR
EXPANSION	SIMPSON STRONG-BOLT 2 (ICC ESR-3037)
CONCRETE SCREW	SIMPSON TITEN HD (ICC ESR-2713)
ADHESIVE	SIMPSON SET-3G (ICC ESR-4057)

ANCHORS SHALL BE INSTALLED IN STRICT CONFORMANCE WITH THE MANUFACTURER'S PRINTED INSTALLATION INSTRUCTIONS AND PRODUCT EVALUATION REPORTS. EMBEDMENTS SPECIFIED ON DRAWINGS ARE ACTUAL EMBEDMENT DEPTHS. DO NOT CUT REINFORCING IN NEW OR EXISTING CONCRETE DURING INSTALLATION. ANCHORS SHALL NOT BE INSTALLED IN POST TENSIONED CONCRETE WITHOUT WRITTEN APPROVAL FROM THE SEOR.

REQUESTS FOR ANCHOR SUBSTITUTIONS SHALL BE SUBMITTED TO THE SEOR IN WRITING ALONG WITH EVIDENCE OF EQUAL OR GREATER CAPACITY UNDER THE SPECIFIC PROJECT CONDITIONS.

ALL-THREAD ROD FOR ADHESIVE ANCHORS SHALL CONFORM TO ASTM F1554 GRADE 55, U.N.O. ANCHORS EXPOSED TO EARTH OR WEATHER SHALL BE PROTECTED FROM CORROSION BY HOT-DIP GALVANIZING OR USE OF STAINLESS STEEL. PERMANENTLY EXPOSED EMBEDDED PLATES AND ANGLES SHALL BE HOT-DIPPED GALVANIZED AFTER FABRICATION, U.N.O.

ADHESIVE ANCHORS SHALL NOT BE INSTALLED FOR A MINIMUM OF 21 DAYS AFTER CASTING CONCRETE IN ACCORDANCE WITH ACI 318-19 SECTION 17.2.2. LOADS SHALL NOT BE PLACED ON ADHESIVE ANCHORS UNTIL ADHESIVE IS FULLY CURED. CONCRETE SUBSTRATE SHALL BE DRY, WITHIN THE ALLOWABLE TEMPERATURE RANGE, HOLES SHALL BE MADE WITH A HAMMER DRILL USING A CARBIDE TIPPED DRILL BIT, AND CLEANED IN ACCORDANCE WITH THE MANUFACTURER'S PRINTED INSTALLATION INSTRUCTIONS.

EXPANSION AND SCREW ANCHORS SHALL NOT BE REMOVED AND RESET. SCREW ANCHORS SHALL NOT BE INSTALLED IN HOLES PREVIOUSLY THREADED BY A PRIOR SCREW ANCHOR.

STRUCTURAL STEEL

STRUCTURAL STEEL SHALL BE OF THE MATERIAL AND TYPE LISTED BELOW, U.N.O.:

STRUCTURAL STEEL	
SHAPE	MATERIAL GRADE
PLATES WHERE NOTED	ASTM A572, GRADE 50
CHANNELS, PLATES AND ANGLES, U.N.O.	ASTM A36
HOLLOW STRUCTURAL SECTIONS (RECTANGULAR)	ASTM A500, GRADE C (Fy=50KSI)

DESIGN, DETAILING, FABRICATION, AND ERECTION SHALL BE IN ACCORDANCE WITH THE AISC 360, "SPECIFICATION FOR STRUCTURAL STEEL BUILDINGS" WITH "COMMENTARY" AND THE "CODE OF STANDARD PRACTICE FOR STEEL BUILDINGS AND BRIDGES", WITH THE FOLLOWING CLARIFICATIONS AND ADDITIONS:

1. CLARIFY SECTIONS 7.5.1 AND 7.5.3 AS FOLLOWS:
   EMBEDMENT LOCATION DRAWINGS SHALL BE SUBMITTED TO THE STRUCTURAL ENGINEER OF RECORD FOR INFORMATION ONLY. THE SEOR IS NOT RESPONSIBLE FOR THE APPROVAL OF EMBEDMENT LOCATION DRAWINGS.
2. ADD THE FOLLOWING PARAGRAPH TO SECTION 7.10.3:
   "THE ERECTOR SHALL HAVE THE SOLE RESPONSIBILITY FOR DETERMINING THE MEANS AND METHODS USED TO PROPERLY AND ADEQUATELY BRACE THE FRAMING DURING ERECTION."

WELDING SHALL CONFORM TO THE AWS CODES FOR ARC AND GAS WELDING IN BUILDING CONSTRUCTION. WELDING SHALL BE PERFORMED IN ACCORDANCE WITH A WELDING PROCEDURE SPECIFICATION (WPS) AS REQUIRED IN AWS D1.1 AND APPROVED BY THE STRUCTURAL ENGINEER. THE WPS VARIABLES SHALL BE WITHIN THE PARAMETERS ESTABLISHED BY THE FILLER-METAL MANUFACTURER.

WELDS SHALL BE MADE USING E70XX ELECTRODES AND SHALL BE 3/16" MINIMUM, UNLESS OTHERWISE NOTED. WELDING SHALL BE BY AWS CERTIFIED WELDERS.

PROVIDE WEEP HOLES AT EXTERIOR CLOSED SECTIONS WHERE MOISTURE MAY ACCUMULATE. LOCATE WEEP HOLES AT BOTTOM OF HORIZONTAL MEMBERS AT MIDSPAN UNLESS OTHER NOTED. LOCATE WEEP HOLES AT BOTTOM OF VERTICAL MEMBERS EXCEPT AT ROOF ASSEMBLIES. ALL WEEP HOLES TO BE APPROVED PRIOR TO FABRICATION.

NON-SHRINK GROUT USED UNDER BEARING AND BASE PLATES SHALL BE ASTM C 1107, FACTORY-PACKAGED, NONMETALLIC AGGREGATE GROUT, NONCORROSIVE, NONSTAINING, MIXED WITH WATER TO CONSISTENCY SUITABLE FOR APPLICATION AND A 30-MINUTE WORKING TIME. GROUT STRENGTH SHALL BE 8,000 PSI MINIMUM AT 28 DAYS.

DISSIMILAR METALS SHALL BE SEPARATED AS REQUIRED TO PREVENT GALVANIC CORROSION BY COMPLETELY COVERING CONTACT AREAS WITH HESKINS 3453 CORROSION PROTECTION TAPE OR APPROVED EQUAL MATERIAL.

GALVANIZING AND DUPLEX COATING

ALL STEEL EXPOSED TO WEATHER OR LOCATED OUTSIDE THE BUILDING ENVELOPE SHALL BE HOT-DIP GALVANIZED UNLESS NOTED OTHERWISE IN PROJECT SPECIFICATIONS OR DRAWINGS. WHERE THESE ELEMENTS ARE ALSO EXPOSED TO VIEW THEY SHALL ADDITIONALLY BE PAINTED OR POWDER COATED PER SPECIFICATIONS AND ARCHITECTURAL DRAWINGS.

CONTRACTOR TO COMMUNICATE WITH GALVANIZER FOR THE PROJECT EARLY ON TO INFORM THE GALVANIZER THAT THE STEEL IS TO RECEIVE A DUPLEX COATING. HOT DIPPED GALVANIZED STEEL THAT IS TO BE PAINTED SHALL BE PREPARED PER ASTM D6386. HOT DIPPED GALVANIZED STEEL THAT IS TO BE POWDER COATED SHALL BE PREPARED PER ASTM D7803.

ALL GALVANIZED STEEL IS TO BE DETAILED TO BE SHOP WELDED AND FIELD BOLTED. WHERE FIELD WELDING IS REQUIRED DUE TO FIELD CONDITIONS, REPAIR DAMAGED GALVANIZED COATING WITH ZINC RICH PAINT PER ASTM A780 WITH EFFECTIVE THICKNESS EQUAL TO HOT-DIP GALVANIZED COATING.

STEEL DECK

STEEL FLOOR DECK SHALL BE A COMPOSITE TYPE WITH RIBS AT 12" o.c. OF THE SIZE AND GAUGE INDICATED ON THE PLANS. STEEL FLOOR DECK SHALL BE VENTED AS REQUIRED BY THE DECK MANUFACTURER BASED ON THE FLOOR COVERING OR COATING BEING USED. STEEL ROOF DECK SHALL BE 1-1/2" TYPE B OR 3" TYPE N (32" WIDTH) OF THE GAUGE INDICATED ON THE PLANS. STEEL DECK SHALL CONFORM TO ASTM A653 DESIGNATION SS, GRADE 50 (Fy = 50 KSI). THE GALVANIZED COATING SHALL CONFORM TO ASTM A653, G60 (G90 WHERE LEFT PERMANENTLY EXPOSED TO WEATHER).

MINIMUM DECK GAUGES INDICATED ON THE PLANS ARE BASED ON 3-SPAN, UN-SHORED CONDITIONS. FOR OTHER SPAN CONDITIONS, DECK MANUFACTURER SHALL EVALUATE AND INDICATE SHORING AS REQUIRED. MINIMUM PROPERTIES SHALL BE AS FOLLOWS:

ROOF DECK PROPERTIES			
DECK TYPE & GAUGE	I (IN^4/FT)	+S (IN^3/FT)	-S (IN^3/FT)
N - 18 GAUGE	1.267	0.652	0.735

WELDED OR MECHANICAL DECK CONNECTIONS SHALL BE PROVIDED AT ALL SUPPORTS AND SIDELAPS AS INDICATED. DECK DESIGN IS BASED ON VERCO IN ACCORDANCE WITH IAPMO ES EVALUATION REPORT ER-0217. EQUAL PRODUCT(S) MAY BE SUBMITTED FOR REVIEW AND APPROVAL BY THE SEOR.

DO NOT HANG OR SUSPEND STRUCTURAL ITEMS DIRECTLY FROM ROOF DECK. NON-STRUCTURAL ITEMS SUSPENDED FROM THE DECK SHALL BE REVIEWED AND APPROVED BY THE ARCHITECT AND STRUCTURAL ENGINEER PRIOR TO INSTALLATION.

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JOB NO: 10022500104
ISSUE DATE: 05/01/2025

GENERAL STRUCTURAL NOTES CONT.

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SPECIAL INSPECTION

GENERAL - SPECIAL INSPECTIONS					
SYSTEM OR MATERIAL	OSSC CODE REFERENCE	CODE OR STANDARD REFERENCE	FREQUENCY (NOTE 6)		REMARKS
			CONTINUOUS	PERIODIC	
FABRICATORS	1705.11 1704.2.5				SPECIAL INSPECTION IS REQUIRED FOR STRUCTURAL LOAD-BEARING MEMBERS AND ASSEMBLIES FABRICATED ON THE PREMISES OF A FABRICATOR'S SHOP. SPECIAL INSPECTIONS SHALL BE PERFORMED DURING FABRICATION. PERFORMING SPECIAL INSPECTIONS IS NOT REQUIRED, WHERE FABRICATOR HAS BEEN APPROVED AS AN APPROVED FABRICATOR, PER SECTION 1704.2.5.1.
POST INSTALLED ADHESIVE ANCHORS WITH SUSTAINED TENSION LOADS INSTALLED HORIZONTALLY OR AT AN UPWARD INCLINE IN HARDENED CONCRETE		ACI 318: 17.2.5	X		Note 8
POST INSTALLED MECHANICAL ANCHORS AND ADHESIVE ANCHORS (EXCLUDING CONDITIONS NOTED ABOVE) IN HARDENED CONCRETE				X	

STATEMENT OF SPECIAL INSPECTION NOTES:

1. SPECIAL INSPECTIONS SHALL CONFORM TO SECTION 1705 OF THE 2022 OSSC, CONTRACT DOCUMENTS AND APPROVED SUBMITTALS. REFER TO SPECIAL INSPECTION AND TESTING TABLES FOR PROJECT REQUIREMENTS.

2. SPECIAL INSPECTIONS AND ASSOCIATED TESTING SHALL BE PERFORMED BY AN APPROVED ACCREDITED INDEPENDENT AGENCY MEETING THE REQUIREMENTS OF ASTM E329 (MATERIALS). THE INSPECTION AND TESTING AGENCY SHALL FURNISH TO THE STRUCTURAL ENGINEER ARCHITECT A COPY OF THEIR SCOPE OF ACCREDITATION. SPECIAL INSPECTORS SHALL BE APPROVED BY THE BUILDING OFFICIAL. WELDING INSPECTORS SHALL BE QUALIFIED PER SECTION 6.1.4.1(1) OF AWS D1.1.

3. THE SPECIAL INSPECTOR SHALL OBSERVE THE INDICATED WORK FOR COMPLIANCE WITH THE APPROVED CONSTRUCTION DOCUMENTS. ALL DISCREPANCIES SHALL BE BROUGHT TO THE ATTENTION OF THE CONTRACTOR FOR CORRECTION AND NOTED IN THE INSPECTION REPORTS.

4. THE SPECIAL INSPECTOR SHALL FURNISH INSPECTION REPORTS FOR EACH INSPECTION TO THE BUILDING OFFICIAL, STRUCTURAL ENGINEER, ARCHITECT, CONTRACTOR, AND OWNER. THE SPECIAL INSPECTION AGENCY SHALL SUBMIT A FINAL REPORT STATING THAT THE WORK REQUIRING SPECIAL INSPECTION WAS INSPECTED AND IS IN CONFORMANCE WITH THE APPROVED CONSTRUCTION DOCUMENTS AND THAT ALL DISCREPANCIES NOTED IN THE INSPECTION REPORTS HAVE BEEN CORRECTED.

5. QUALITY ASSURANCE (QA) IS REQUIRED FOR STRUCTURAL STEEL ITEMS PER AISC 360 AND 341 UNLESS SPECIFICALLY NOTED OTHERWISE. QUALITY CONTROL (QC) TO BE PROVIDED BY THE FABRICATOR, ERECTOR OR OTHER RESPONSIBLE CONTRACTOR AS APPLICABLE. CONTRACTOR AND SPECIAL INSPECTOR TO DOCUMENT QUALITY CONTROL AS REQUIRED IN AISC 360 SECTION N3 AND AISC 341 SECTION J2.

6. **INSPECTION TYPES:**
   CONTINUOUS : THE FULL-TIME OBSERVATION OF WORK REQUIRING SPECIAL INSPECTION BY AN APPROVED SPECIAL INSPECTOR WHO IS PRESENT IN THE AREA WHERE THE WORK IS BEING PERFORMED.
   PERIODIC : THE PART-TIME OR INTERMITTENT OBSERVATION OF WORK REQUIRING SPECIAL INSPECTION BY AN APPROVED SPECIAL INSPECTOR WHO IS PRESENT IN THE AREA WHERE THE WORK HAS BEEN OR IS BEING PERFORMED AND AT THE COMPLETION OF THE WORK.
   OBSERVE : OBSERVE THESE FUNCTIONS ON A RANDOM, DAILY BASIS. OPERATIONS NEED NOT BE DELAYED PENDING OBSERVATIONS.
   PERFORM : INSPECTIONS SHALL BE PERFORMED PRIOR TO THE FINAL ACCEPTANCE OF THE ITEM.

7. PERFORM INSPECTION PRIOR TO FINAL ACCEPTANCE OF THE ITEM FOR TEN WELDS TO BE MADE BY A GIVEN WELDER, WITH THE WELDER DEMONSTRATING UNDERSTANDING OF REQUIREMENTS AND POSSESSION OF SKILLS AND TOOLS TO VERIFY THESE ITEMS, THE PERFORM DESIGNATION OF THIS TASK SHALL BE REDUCED TO OBSERVE, AND THE WELDER SHALL PERFORM THIS TASK. SHOULD THE INSPECTOR DETERMINE THAT THE WELDER HAS DISCONTINUED PERFORMANCE OF THIS TASK, THE TASK SHALL BE RETURNED TO PERFORM UNTIL SUCH TIME AS THE INSPECTOR HAS RE-ESTABLISHED ADEQUATE ASSURANCE THAT THE WELDER WILL PERFORM THE INSPECTION TASKS LISTED.

8. SPECIAL INSPECTION OF MECHANICAL POST INSTALLED ANCHORS SHALL BE IN STRICT CONFORMANCE WITH THE ICC REPORT AND MANUFACTURER'S INSTALLATION REQUIREMENTS. ANCHOR INSTALLERS SHALL BE QUALIFIED AS REQUIRED BY JURISDICTION REQUIREMENTS.
   - INSPECTION REPORTS SHALL IDENTIFY NAMES OF INSTALLERS.
   - SPECIAL INSPECTOR SHALL PROVIDE DOCUMENTATION AT THE END OF ANCHOR INSTALLATIONS STATING THAT THE ANCHORS WERE INSPECTED PER APPROVED ANCHOR EVALUATION REPORT.

SOILS/GEOTECHNICAL - SPECIAL INSPECTIONS					
SYSTEM OR MATERIAL	OSSC CODE REFERENCE	CODE OR STANDARDS REFERENCE	FREQUENCY (NOTE 6)		REMARKS
			CONTINUOUS	PERIODIC	
SOILS					
VERIFY MATERIALS BELOW SHALLOW FOUNDATIONS ARE ADEQUATE TO ACHIEVE THE DESIGN BEARING CAPACITY	1705.6	GEOTECHNICAL REPORT		X	BY THE GEOTECHNICAL ENGINEER OR QUALIFIED SPECIAL INSPECTOR
VERIFY EXCAVATIONS ARE EXTENDED TO PROPER DEPTH AND HAVE REACHED PROPER MATERIAL				X	
PERFORM CLASSIFICATION AND TESTING OF COMPACTED FILL MATERIALS				X	
DURING FILL PLACEMENT, VERIFY USE OF PROPER MATERIALS AND PROCEDURES IN ACCORDANCE WITH THE PROVISIONS OF THE APPROVED GEOTECHNICAL REPORT. VERIFY DENSITIES AND LIFT THICKNESSES DURING PLACEMENT AND COMPACTION OF COMPACTED FILL			X		
PRIOR TO PLACEMENT OF COMPACTED FILL, INSPECT SUBGRADE AND VERIFY THAT SITE HAS BEEN PREPARED PROPERLY				X	

CONCRETE - SPECIAL INSPECTIONS					
SYSTEM OR MATERIAL	OSSC CODE REFERENCE	CODE OR STANDARD REFERENCE	FREQUENCY (NOTE 6)		REMARKS
			CONTINUOUS	PERIODIC	
GENERAL	1705.3 1901.6	ACI 318: 26.13			SPECIAL INSPECTIONS OF CONCRETE SHALL CONFORM TO THE REQUIREMENTS OF SECTION 1705.3 OF THE IBC AND SECTION 26.13 OF ACI 318.
REINFORCING STEEL PLACEMENT	1901.5	ACI 318: CH. 20, 25.2, 25.3, 26.6.1-26.6.3, 26.13.3.3		X	REINFORCING TO COMPLY WITH ALL CODE PROTECTION, SPACING AND TOLERANCE LIMITS.
INSPECT ANCHORS/BOLTS CAST IN CONCRETE	-	ACI 318: 26.13.3.3(g)		X	ALL CAST-IN-PLACE ANCHORS/BOLTS SHALL BE VISUALLY INSPECTED. REFERENCE STEEL INSPECTIONS FOR ADDITIONAL INSTALLATION, MATERIAL AND WELDING INSPECTIONS OF STEEL ITEMS EMBEDDED IN CONCRETE (HEADED STUDS, DBA's, ETC.)
VERIFYING USE OF REQUIRED MIX DESIGN(S)	1904.1 1904.2	ACI 318: CH. 19, 26.4.3, 26.4.4		X	
CONCRETE SPECIMENS FOR TESTING		ASTM C172 ASTM C31 ACI 318: 26.5, 26.12	X		PRIOR TO CONCRETE PLACEMENT, FABRICATE CONCRETE SPECIMENS FOR TESTING. SEE THE CONCRETE TESTING TABLE FOR ADDITIONAL INFORMATION.
CONCRETE PLACEMENT		ACI 318: 26.5, 26.13.3.2(a)	X		
CONCRETE CURING		ACI 318: 26.5.3 - 26.5.5, 26.13.3.3		X	VERIFY MAINTENANCE OF SPECIFIED CURING TEMPERATURES AND TECHNIQUES
VERIFICATION OF FORMWORK		ACI 318: 26.11.1.2(b), 26.13.3.3		X	SPECIAL INSPECTIONS APPLY TO SHAPE, LOCATION AND DIMENSIONS OF THE CONCRETE MEMBER BEING FORMED

CONCRETE - TESTING				
SYSTEM OR MATERIAL	OSSC CODE REFERENCE	CODE OR STANDARD REFERENCE	FREQUENCY (NOTE 6)	REMARKS
CONCRETE STRENGTH	1705.3 ASTM C172 ASTM C 31 ACI 318 26.12 ACI 318 26.5	ASTM C39	EACH 150 CY NOR LESS THAN EACH 5000 SF OF SLAB OR WALL PLACED EACH SHIFT	FABRICATE SPECIMENS AT TIME FRESH CONCRETE IS PLACED
CONCRETE SLUMP		ASTM C143		
CONCRETE AIR CONTENT		ASTM C231		
CONCRETE TEMPERATURE		ASTM C1064		

STEEL - SPECIAL INSPECTIONS					
SYSTEM OR MATERIAL	OSSC CODE REFERENCE	CODE OR STANDARD REFERENCE	INSPECTION (NOTES 5 AND 6)		REMARKS
			CONTINUOUS/ PERFORM	PERIODIC/ OBSERVE	
STEEL FABRICATION					
FABRICATION OF STRUCTURAL ELEMENTS	1704.2.5.1	AISC 360		X	REFER TO INSPECTION OF FABRICATOR REQUIREMENTS
MATERIAL VERIFICATION OF STRUCTURAL STEEL COMPONENTS	1705.2.1 TABLE 1705.2-3	ASTM A6 ASTM STANDARDS SPECIFIED IN CONSTRUCTION DOCUMENTS AISC 360 A3.1 AISC 360 N2.1 AISC 360 N3.2		X	CERTIFIED MILL TEST REPORTS IDENTIFICATION MARKINGS TO CONFORM TO AISC 360 AND ASTM STANDARDS SPECIFIED IN THE APPROVED CONSTRUCTION DOCUMENTS
MATERIAL VERIFICATION OF HIGH STRENGTH BOLTS, NUTS, AND WASHERS	1705.2.1.1 AISC 360 N5 TABLE 1705.2-1	AISC 360 A3.3 AISC 360 N3.2 ASTM STANDARDS SPECIFIED IN CONSTRUCTION DOCUMENTS RCSC 2.1		X	MANUFACTURER'S CERTIFIED TEST REPORTS IDENTIFICATION MARKINGS TO CONFORM TO ASTM STANDARDS AND SPECIFIED IN THE APPROVED CONSTRUCTION DOCUMENTS MANUFACTURER'S CERTIFICATE OF COMPLIANCE
MATERIAL VERIFICATION OF ANCHOR BOLTS AND THREADED RODS		AISC 360 A3.4 AISC 360 N3.2 ASTM STANDARDS SPECIFIED IN CONSTRUCTION DOCUMENTS		X	MANUFACTURER'S CERTIFIED TEST REPORTS
MATERIAL VERIFICATION OF WELD FILLER METALS	1705.2.1.1 TABLE 1705.2-5	AISC 360 A3.5 AISC 360 N3.2 APPLICABLE AWS A5 DOCUMENTS		X	MANUFACTURER'S CERTIFICATE OF COMPLIANCE IDENTIFICATION MARKINGS TO CONFORM TO AWS SPECIFICATION IN THE APPORVED CONSTRUCTION DOCUMENTS
STRUCTURAL STEEL WELDING					
VERIFYING USE OF PROPER WPS'S	1705.2.1 AWS D1.1	AISC 360 N3.2			RETAIN A RECORD OF WELDING PROCEDURE SPECIFICATIONS
VERIFYING WELDER QUALIFICATIONS		AWS D1.1		X	RETAIN A RECORD OF QUALIFICATION CARDS
COMPLETE AND PARTIAL JOINT PENETRATION GROOVE WELDS	TABLE 1705.2-6	AWS D1.1 CLAUSE 6	X		
MULTIPASS FILLET WELDS			X		
SINGLE PASS FILLET WELDS GREATER THAN 5/16"			X		ALL WELDS VISUALLY INSPECTED PER AWS D1.16.9
PLUG AND SLOT WELDS			X		
SINGLE PASS FILLET WELDS LESS THAN OR EQUAL TO 5/16"				X	

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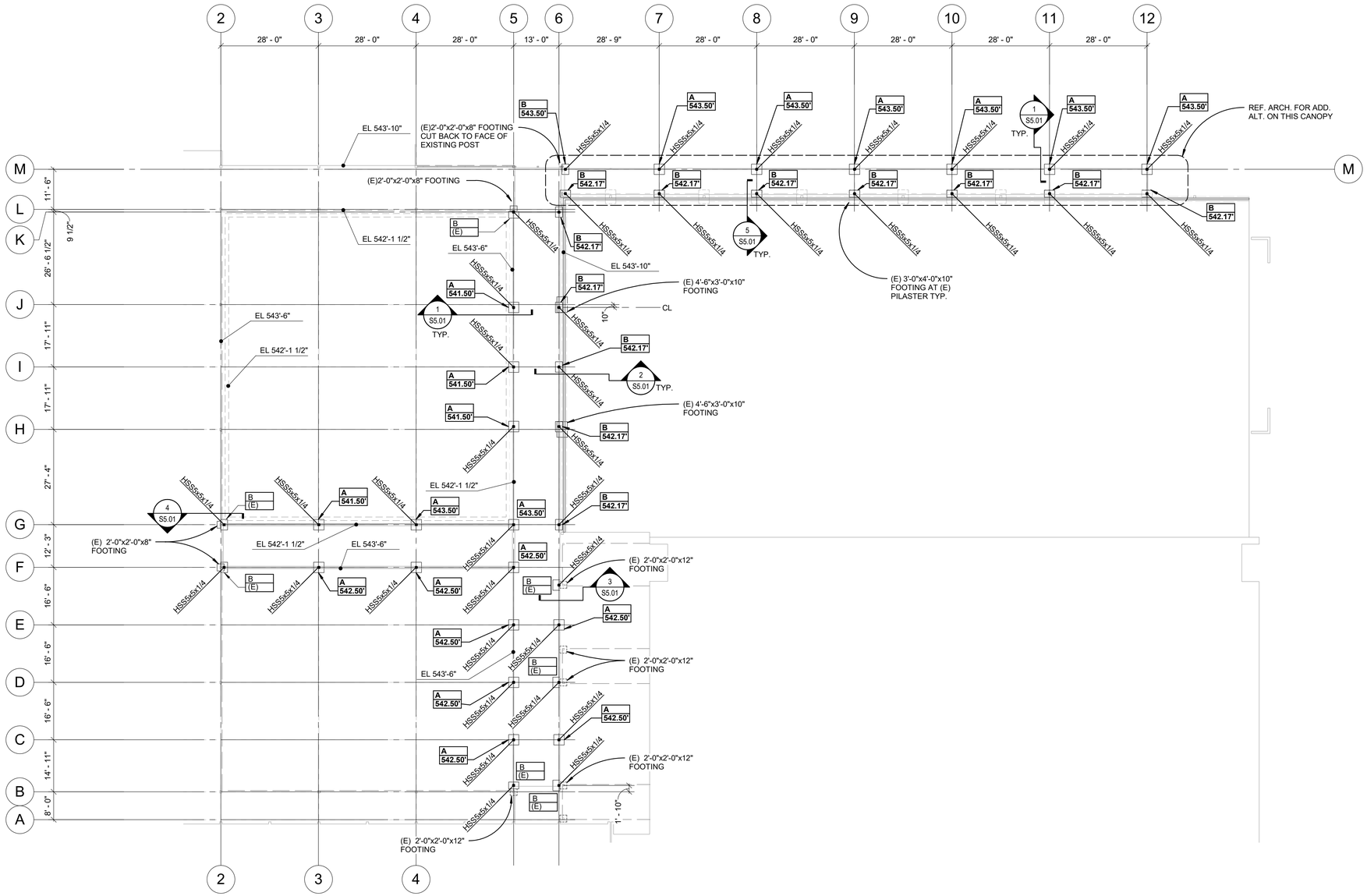
SPECIAL INSPECTION

S0.04



FOUNDATION PLAN NOTES:

1. X / -XX.XX' — INDICATES FOOTING TYPE. REF. PLAN FOR SCHEDULE. INDICATES TOP OF FOOTING ELEVATION. (E) INDICATES MATCHING BOTTOM OF EXISTING FOOTING.
2. EL. XXX'-XX" — INDICATES TOP OF STRUCTURAL SLAB ELEVATION. REF. ARCHITECTURAL SLAB DIMENSION PLANS FOR TOP OF OVERFRAMING SLABS.
3. ——— — INDICATES EXISTING STRUCTURE.
4. FIELD VERIFY EXISTING DIMENSIONS AND ELEVATIONS.
5. REF. ARCHITECTURAL SLAB DIMENSION PLANS FOR DIMENSIONS NOT SHOWN.
6. ALL EXTERIOR STEEL TO BE GALVANIZED.

SPREAD FOOTING SCHEDULE

MARK	SIZE "A"	SIZE "B"	"T"	REINFORCING
A	3' - 0"	3' - 0"	1' - 0"	(4) #4 EACH WAY TOP AND BOTTOM
B	2' - 0"	3' - 0"	1' - 0"	(4) #4 EACH WAY TOP AND BOTTOM

1 CANOPY FOUNDATION PLAN
1/16" = 1'-0"

N

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CRESWELL HS CANOPY REPLACEMENT

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FOUNDATION PLAN

S1.10

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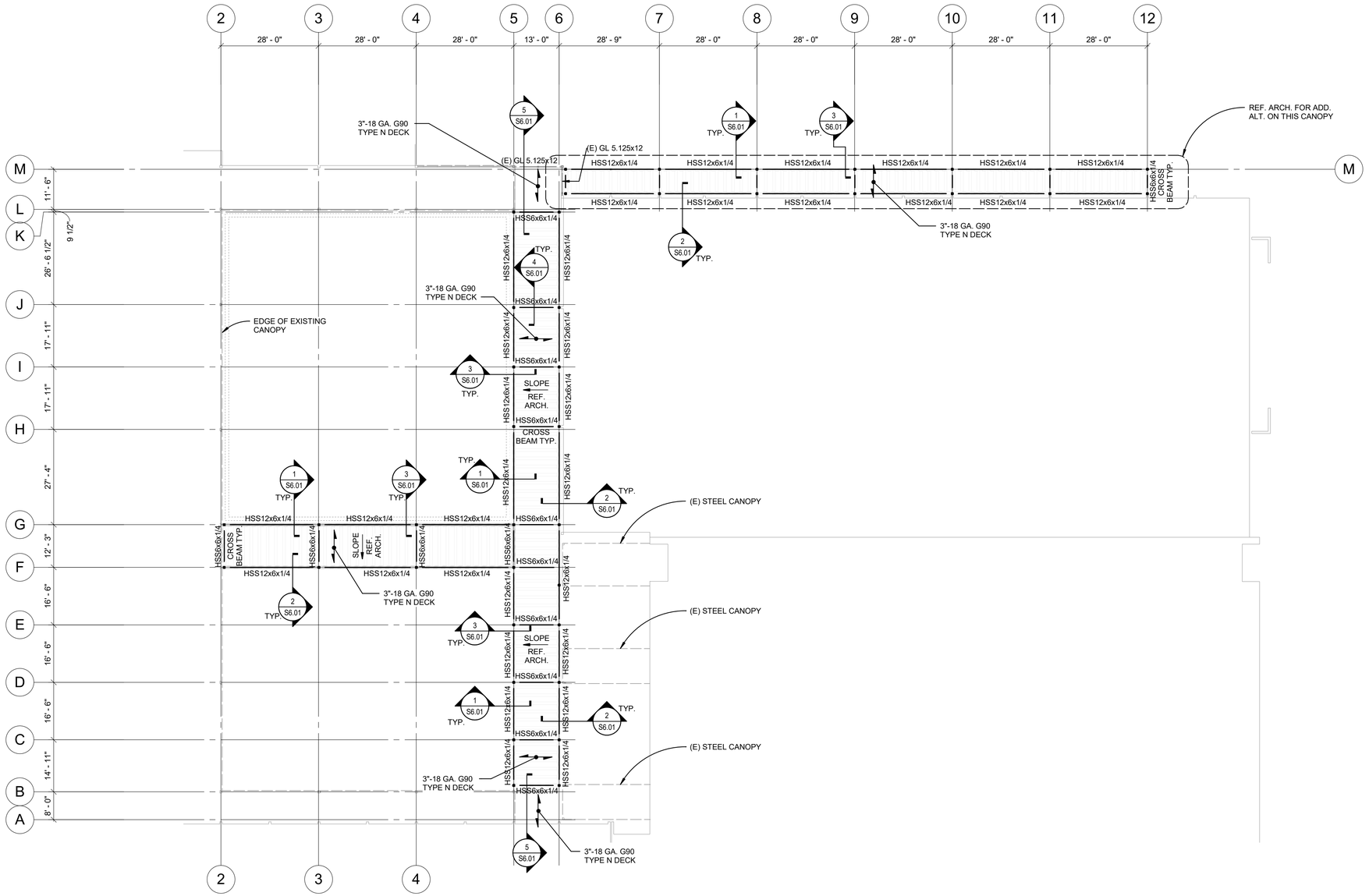

1 CANOPY FRAMING PLAN

1/16" = 1'-0"

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10022500104 - R25

REVISIONS

CRESWELL HS CANOPY REPLACEMENT

33390 NIBLOCK LN, CRESWELL, OR 97426
CONSTRUCTION DOCUMENTS

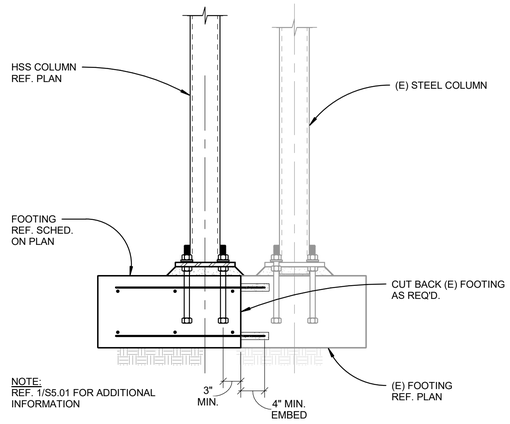
JOB NO: 10022500104

ISSUE DATE: 05/01/2025

CANOPY FRAMING PLAN

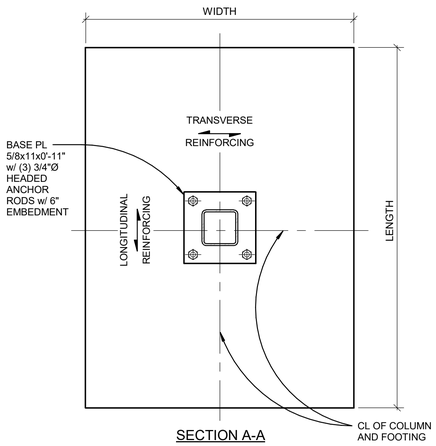
S1.11

9/1/2025 2:32:14 PM
Revit Model: 10022500104_Creswell HS.rvt

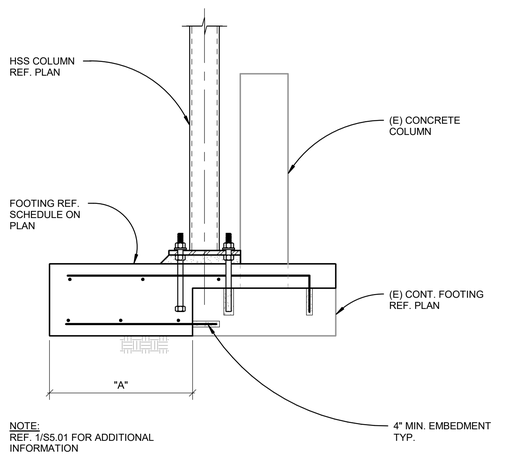


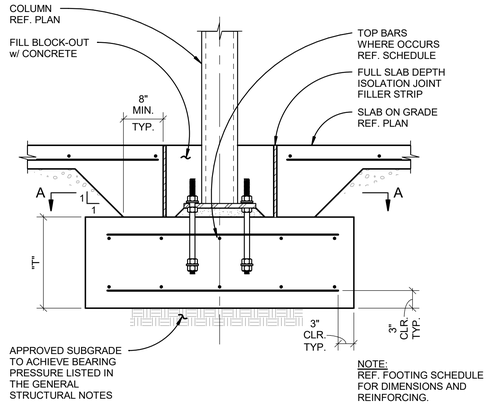
3 FOOTING AT EXISTING FOOTING

1" = 1'-0"



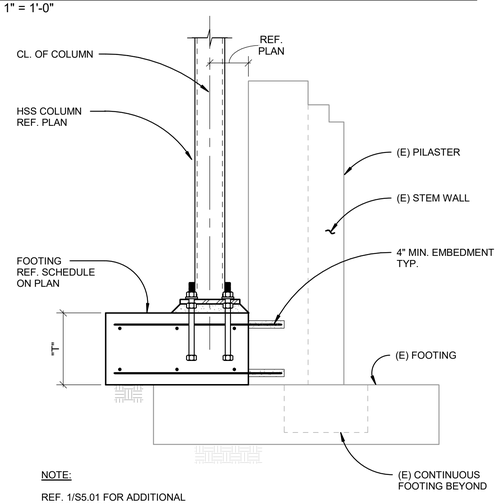
SECTION A-A





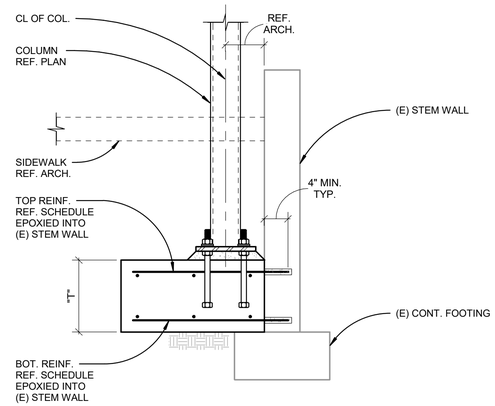
4 FOOTING AT EXISTING FOOTING

1" = 1'-0"



1 TYP. FOOTING AT STEEL COLUMN

1" = 1'-0"



5 FOOTING AT PILASTER DETAIL

1" = 1'-0"

2 FOOTING AT EXISTING FOOTING

1" = 1'-0"

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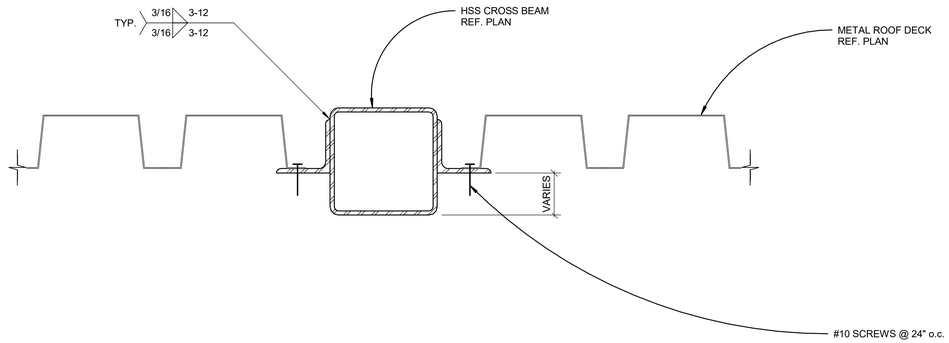
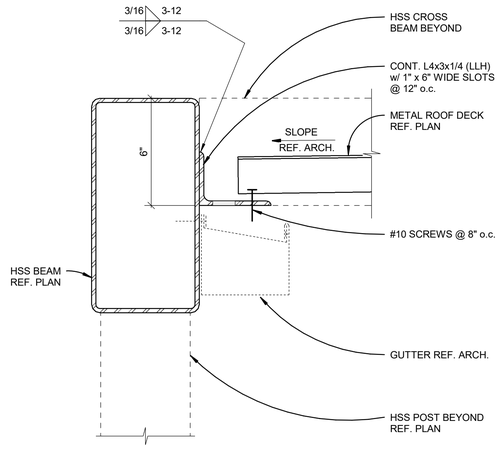
CONCRETE DETAILS

S5.01



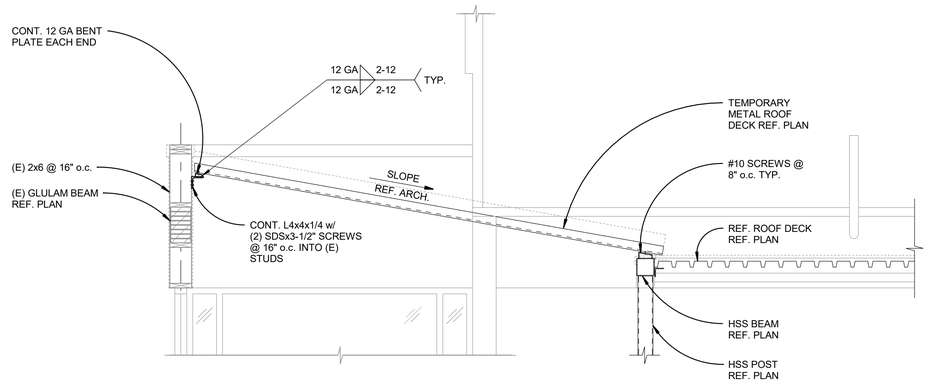
4 DECK SUPPORT AT CROSS BEAM

3" = 1'-0"



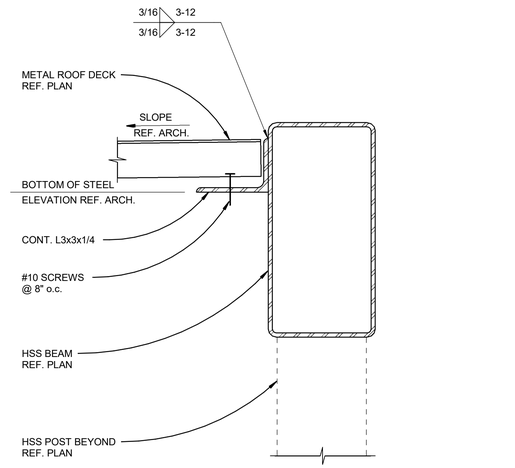
1 DECK SUPPORT AT LOW END

3" = 1'-0"



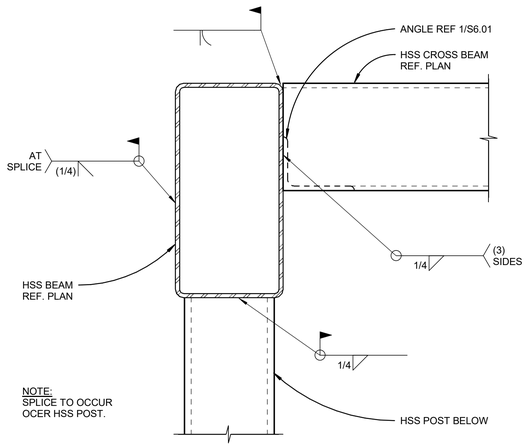
5 TEMPORARY METAL DECK FRAMING

1/2" = 1'-0"



2 TYPICAL TOP RAKE DETAILT

3" = 1'-0"



3 BEAM TO COLUMN CONNECTION

3" = 1'-0"

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STEEL DETAILS

S6.01